PRELIMINARY REPORT

for

BOREHAM CONSULTING ENGINEERS LTD

on behalf of

JELSON LIMITED

on

A GROUND INVESTIGATION

for

A PROPOSED RESIDENTIAL DEVELOPMENT

at

HALLAM FIELDS BIRSTALL LEICESTERSHIRE

Nicholls Colton Geotechnical Consulting, Inspecting and Testing Engineers 7-11 Harding Street Leicester LE1 4DH

LR: G04003 DATE: December 2003

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PRELIMINARY REPORT ON A PROPOSED DEVELOPMENT AT HALLAM FIELDS, BIRSTALL, LEICESTERSHIRE.

1. **Introduction**

- 1.1 This preliminary report has been prepared on instructions given by Boreham Consulting Engineers Ltd, Regent House, Hubert Road, Brentwood, Essex, CM14 4JE on behalf of their clients Jelson Limited, 370 Loughborough Road, Leicester, LE4 5PR in a letter dated 21st August 2003.
- 1.2 It is proposed that approximately 900 residential properties with associated infrastructure be developed at the site.
- 1.3 The purpose of this ground investigation was to obtain preliminary data regarding the ground and groundwater conditions at the site, so that suitable methods of design and construction can be adopted.
- 1.4 The site is located West of the A6 directly north of Birstall, Leicestershire (approximate National Grid Reference SK 587 103) as shown on the appended site location plan (Nicholls Colton Geotechnical Drawing No. G04003/01).
- 1.5 This preliminary investigation has been undertaken on the specific instructions of Boreham Consulting Engineers Ltd on behalf of their clients Jelson Ltd and no reliance should be made by third parties as to its adequacy or completeness in relation to future development requirements of the area under consideration. Similarly, Nicholls Colton Geotechnical and Nicholls Colton and Partners Ltd will accept no liability in relation to the use of data contained in this report by third parties.

2. **Site Description**

- 2.1 The area of the site under investigation predominantly consists of four arable fields separated by hedgerow, which together form a large rectangular shape.
- 2.2 A small pond is located in the centre of the site, within a line of hedgerow.
- 2.3 The site is slopes down in a south-east direction with an approximate fall of some 25m. At the time of the site works the ground was uneven, having been ploughed.
- A water pipe (1.5 inch asbestos cement) is known to cross the site in an east-west direction. There are no other known services upon the site.
- 2.5 Access to the site was gained from the A6 via a farm gate on the east side of the site.

3. **Site Work**

- 3.1 Eight 150mm diameter boreholes (BH's 1 to 8) were put down by light cable percussion methods to depths of 8.00m below existing ground level. Water monitoring pipes were installed in all of the boreholes to full depth.
- 3.2 Fifty trial pits (TP's 1 to 50) were excavated by mechanical excavator to depths of between 1.60m and 3.30m (although predominantly to 3.00m) below existing ground level. All trial pits were backfilled with arisings upon completion.
- In addition, three soakaway pits (SK's 1, 2 and 3) were excavated by mechanical excavator to depths of between 2.10m and 2.20m below existing ground level. These pits were backfilled with 20mm (nominal size) gravel over the proposed tested zone (1.00m to full depth) with a monitoring / filling pipe installed to full depth. These pits were then backfilled with arisings to ground level.
- 3.3 The Borehole, trial pit and soakaway pit logs are included in this report as Appendices A, B and C respectively.
- 3.4 The positions of the exploratory holes (including soakaway test pits) are shown on the appended exploratory hole location plan (Nicholls Colton Geotechnical Drawing No. G04003/02) based on a drawing supplied by Boreham Consulting Engineers Ltd.
- 3.5 Open tube samples (105mm nominal diameter) were recovered from the cohesive strata revealed within the cable percussion boreholes. Fragmentary disturbed samples were recovered from all materials revealed within the boreholes and machine excavated trial pits.
- 3.6 The samples were taken to Nicholls Colton Testing's laboratory for examination and testing.
- 3.7 The fieldwork (excluding soakaway testing) was carried out between 17th October and 8th November 2003.
- 3.8 This investigation has been carried out in general accordance with BS 5930: 1999.

4. In Situ Testing

- 4.1 Standard Penetration Tests (SPT's) were carried out within the natural granular and cohesive materials revealed in the boreholes. The tests were carried out in accordance with BS 1377: Part 9: 1990 and the results are included on the appended cable percussion borehole logs (Appendix A).
- 4.2 Hand shear vane tests were performed where practicable in cohesive strata within the trial pits. The results are included on the trial pit logs (Appendix B).

- 4.3 Mexicone tests (CBR equivalent) were undertaken within the majority of the trial pits at anticipated approximate road formation level. The results are included on the trial pit logs (Appendix B).
- 4.4 As described in section 3.3 three soakaway pits were constructed, in order to determine the drainage characteristics of the soils at the site. These pits were filled (to 1.00m below ground level) with potable water. The water level was then measured until the pit was empty. This process was undertaken three times per pit. It was not possible to complete the test for SK3 as the water from the first stage did not drain away.
- 4.5 The results of the soakaway testing are presented in Appendix D.
- 4.6 The soakaway testing was carried out from 22nd to 27th November 2003.

5. **Laboratory Testing**

- 5.1 The laboratory testing schedule was prepared by Nicholls Colton Geotechnical, based on quantities determined by Boreham Consulting Engineers Ltd.
- 5.2 Included in this report is information taken from the results of tests undertaken in Nicholls Colton Testing's laboratory at 7 11 Harding Street, Leicester. This is UKAS accredited testing laboratory No. 0320. The results of the laboratory tests in this report do not include all data required by the documented test procedure. However, all such data has been recorded and will be stored for six years from the date of test. This data will be issued on the client's instructions.

The following accredited test procedures were carried out:

- Natural Moisture Content
- Liquid Limit
- Plastic Limit
- Plasticity Index

- Soil Sulphate Content
- pH value
- California Bearing Ratio
- Particle Size Distribution
- Undrained Triaxial Compression Strength (Single-stage)

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation.

- The geotechnical laboratory tests were carried out during the period 16th October to 14th November 2003.
- The geotechnical laboratory testing has been carried out in accordance with BS 1377: 1990 using calibrated equipment specified within the British Standard.

- 5.5 The geotechnical laboratory test results are included in this report as Appendix E.
- 5.6 In addition to the laboratory testing outlined above a series of soil samples was tested for the presence of contaminants as listed below. This testing was undertaken at UKAS accredited testing laboratory No. 1510 (City Analytical Services, 80 Lockhurst Lane, Coventry, CV6 5PZ):
- 5.7 Concentrations of the following contaminants (based on a suite specified by Boreham Consulting Engineers Ltd) were determined using selected soil samples obtained from the site (all values total unless otherwise stated):

• Arsenic

• Boron (water soluble)

Cadmium

• Chromium

• Copper

• Cyanide

• Lead

• Mercury

Nickel

• pH Value

• Phenols

• Selenium

• Sulphate

• Sulphide

• Sulphur (elemental)

• Zinc

• Polyaromatic Hydrocarbons

- 5.8 The contamination testing outlined above was carried out in the period 13th to 22nd October 2003.
- 5.9 The contamination test results are included in this report as Appendix F.

6. General Geology and Revealed Strata

6.1 Geological sources (including the Geological Survey of Great Britain (England and Wales) sheet no. 156) indicate the site to lie in an area where Glacial Deposits of Pleistocene Age overlie Mercia Mudstone Deposits of Triassic Age.

The exploratory holes revealed a general downwards strata succession of:

TOPSOIL

GLACIAL DEPOSITS - Sli

Slightly gravelly CLAY

Slightly silty slightly gravelly SAND

- SILT

7. **Groundwater**

7.1 Groundwater was encountered in only two of the excavations. A slight seepage was observed in BH6 from 4.50m which rose to 4.30m after 20 minutes. A slight seepage was also observed in TP 4, coming from a pocket of sand within the pit. No other groundwater seepages were encountered.

- 7.2 No long term monitoring of groundwater levels has been undertaken as part of this investigation.
- 7.3 It should be noted that groundwater levels may differ at times from those reported due to seasonal variations or other effects.

ENGINEERING ASSESSMENT AND RECOMMENDATIONS

8. **Introduction**

8.1 It is understood that Jelson Limited propose to construct approximately 900 houses with associated infrastructure at the site. It has been assumed that this development will generally comprise two storey structures.

9. **Soil Profile**

9.1 **Topsoil**

9.1.1 Topsoil was revealed in all of the exploratory holes put down at the site to depths of between 0.10m and 0.60m (although generally to about 0.30m) below existing ground level.

9.2 Glacial Deposits

- 9.2.1 Underlying the topsoil Glacial Deposits were encountered in all locations. These natural soils were encountered as generally stiff slightly gravelly clays and slightly gravelly sands.
- 9.2.2 The granular deposits were generally seen in the south eastern corner of the site although localised pockets were encountered across the site.
- 9.2.3 A localised pocket of non cohesive silt was encountered in TP 27 extending between 0.60m and 2.80m below existing ground level.
- 9.2.4 The granular deposits are considered to be generally of medium dense relative density. This was assessed visually from the ease of excavation and the stability of the pits.
- 9.2.5 The cohesive deposits are typically of stiff or very stiff consistency. They are considered to be of generally medium compressibility.

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- 9.2.6 Seven plasticity index tests undertaken in the cohesive deposits indicate them to be of Low and Intermediate Plasticity (i.e. CL and CI soils) after the classification system of BS 5930: 1999, and to be of low and medium swelling/shrinkage potential in accordance with NHBC Standards Chapter 4.2: 1999.
- 9.2.7 The full depth of the Glacial Deposits was not proven in any of the excavations undertaken.

10. Excavations

- 10.1 Based on the ground conditions revealed during the fieldwork it is anticipated that shallow excavations (less than say 3.00m) should generally be readily achieved adopting standard excavation plant.
- 10.2 Random and potentially severe falls should be anticipated from the faces of near vertically sided unsupported excavations carried out in the natural granular strata at the site. It is recommended that where personnel are required to enter excavations in such materials, continuous support should be provided to the full depth of the excavation. A reduction in the degree of support required (to intermittent support) may be justifiable in the stiff clays.
- Battering of excavation faces to a safe angle as detailed in BS 8000: Part 1:1989 may prove to be a viable alternative to support methods.
- 10.4 All battering or support systems provided should be continually assessed by fully trained and experienced personnel.
- Based on the limited groundwater observations recorded during the fieldwork, it is anticipated that excavations for foundation and drainage systems will generally remain dry.

11. **Foundations**

- 11.1 It is proposed to construct approximately 900 houses (assumed to be two storey) at the site.
- 11.2 The exploratory holes have revealed depths of up to 0.60m of topsoil across the site which is underlain by stiff clays and medium dense sands. It is considered that these deposits should provide foundations with adequate support. It is therefore anticipated that traditional strip or trench fill foundations will prove suitable for new buildings at the site.

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- 11.3 As a guide, for 0.60 metre wide strip/trench fill footings, bearing on the underlying natural stiff cohesive soils at a minimum depth of 0.90 metre below finished ground level, an allowable net bearing pressure of at least 125 kN/m² is considered appropriate.
- 11.4 Deepening of footings below the minimum of 0.90 metre will be required for plots within influencing distance of any trees or vegetation, in accordance with NHBC Standards Chapter 4.2: 1999 'Building near trees'. For the purposes of using that document, and based upon the data obtained during the current investigation, it is recommended that the soils be classified as being of medium swelling/shrinkage potential.
- Where footings are to bear on the natural granular soils (south eastern corner of site) a minimum depth of 0.75m can be adopted. An allowable net bearing pressure of about 125 kN/m² would be appropriate, although it should be noted that in BH5, a loose sand was encountered. Foundations in this area should be deepened (to about 2.00m) and/or widened to minimise the potential for excessive settlements.
- 11.6 Where footings are to bear either in whole or in part on granular soils, it is recommended that these incorporate nominal mesh reinforcement, in order to minimise the potential for differential movements.
- 11.7 The above values incorporate a factor of safety of 3 against shear failure and total settlements are not expected to exceed 20mm, thereby keeping differential settlements within acceptable limits.

12. **Ground Floor Slabs**

- 12.1 Following removal of all topsoil from below the proposed slab a ground bearing slab may be adopted, providing that the thickness of imported granular fill required to achieve slab formation level is less than 600mm.
- Where plots are within influencing distance of trees, it may be necessary to construct ground floor slabs as suspended slabs, in accordance with NHBC standards.
- 12.3 The BRE report 'Radon: guidance on protective measures for new dwellings' 1999 indicates that no radon protection will be required at the site. Furthermore, it specifies that a confirmatory report from the British Geological Society will not be required.

13. Concrete Classification

The guidelines given in BRE Special Digest 1, Part 1:2001 are based upon a site classification relating to its previous usage. This site has been determined to be a greenfield site (natural soils) for the purpose of concrete classification.

- The results of water soluble sulphate tests lie in the range <0.10 to 0.10 g/l as SO₄, and the pH values of soil samples lie within the range 7.1 to 8.4.
- On the basis of the above it is considered appropriate to adopt a Basic Design Sulphate Class of DS-1, together with an Aggressive Chemical Environment for Concrete (ACEC) of AC-1s.

14. **Road Construction**

- 14.1 Natural stiff clays and medium dense sands are generally present at the anticipated formation level (understood to be 0.50m to 0.60m below existing ground level).
- On the basis of the insitu (mexicone) testing and laboratory test results obtained (see Appendices B and E respectively), it is considered appropriate to adopt a design CBR value of 5% at the proposed formation level at the site.
- 14.3 The approval of the local authority Highway Department should be sought in respect of the above.

15. **Drainage**

- 15.1 As described in section 4.4 three soakaway tests were undertaken in order to determine the drainage characteristics of the soils at the site. These tests were carried out in accordance with BRE Digest 365:1991. The results of these tests are presented in Appendix D.
- 15.2 Pits SK 1 and 2 were constructed within the granular deposits encountered in the south-eastern third of the site, whilst SK 3 was constructed within the cohesive deposits encountered in the remaining north-western area. The approximated extent of the clay/sand is shown on the attached plan.
- The positions of the soakaway pits are shown on the Exploratory Hole location plan (G04003/02).
- 15.4 Soakaway pits SK 1 and 2, located in the granular soils had infiltration rates of 1.23x10⁻⁶ m/s and 1.14x10⁻⁵ m/s respectively. These are considered to be 'good' infiltration rates which will be suitable to adopt a soakaway, based on BRE Digest 365:1991.
- 15.5 Soakaway pit SK 3 located in the clay had an infiltration rate of 7.11x10⁻⁸m/s. This is considered to be 'practically impervious' and hence will be unsuitable to adopt a soakaway, based on BRE Digest 365:1991.

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15.6 It should be appreciated that despite the tests in the granular soils indicating good infiltration rates it is possible that the water table at the site may be higher in the wetter winter months. This may affect the suitability of soakaways at the site.

16. **Contamination Assessment**

- 16.1 Ten soil samples were initially tested to determine the presence, if any, of various potential contaminants. The locations and the quantities of contamination testing were specified by Boreham Consulting Engineers Ltd.
- The DEFRA and Environment Agency Contaminated Land Exposure Assessment (CLEA) Model, together with a series of supporting Contaminated Land Reports (CLR) and associated documents provides guidance on the assessment of potentially contaminated sites.
- 16.3 These documents contain Soil Guideline Values (SGV's). Exceedance of an SGV implies that an unacceptable human health risk may be present, and therefore either further site specific risk assessment is required, or intervention (remedial) works are necessary. The CLEA Model includes only selected contaminants at present and therefore the SGV's are only available for: arsenic, cadmium, chromium, lead, mercury, nickel and selenium.
- SGV's are set according to the proposed end use of the land. For the purposes of this report, 'residential housing with plant uptake' land use figures have been adopted.
- 16.5 The results indicate that the near surface soils present at the site are not contaminated. All of the potential contaminants were recorded at levels below the appropriate 'SGV's. For those contaminants not covered by the published SGV's, it is considered by inspection that these are not present at levels that might be of concern. This is consistent with the greenfield nature of the site.
- 16.6 It should be noted that the above is based on limited number of tests and test locations. The local Environmental Health Authority and/or the NHBC may require further confirmatory evidence of the contamination status of the site.
- 16.7 It is recommended that the results of the testing be submitted to the local Environmental Health Department to ensure any specific guidelines, requirements or comments which they may have are addressed.

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APPENDIX A

BOREHOLE LOGS - CABLE PERCUSSION

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NICHOLI Tel: 0116		OLTON AI 36333	ND PA	RTNERS	S LTD.	BOREHOLI (Cable Pe		Borel Num	
Site Hallam	Fiel	ds, Birsta	all Le	icester	chire			Bł	- 11
Client	1 101	<u>ao, Birot</u>	an, E0	1000101	011110.	Boring diameter:	Casing diameter:	Logged k	ov. BE
Jelson	Limi	ted.				150 mm to 8.00m	150 mm to 3.00m	Scale:	1:50
LR:G040			nd Lev	el:		Date: 17/09/2003	Location: -	Sheet	
Sam	ples &		Water	Level	Depth	Descrip		Legend	Install-
Depth (m) 0.00-0.20 0.20-0.90	Type D 1 D 2	SPT N		(mAOD)	(m) 0.20	Mixed very stiff brown slig with some rootlets and TO			ation
0.90-1.15 1.20-1.65		(25)			0.90	Very stiff orange brown s gravelly CLAY. Gravel is to sub-rounded chalk, qua flint.	lightly sandy slightly fine to coarse angular artz, siltstone and		
1.70 1.70 1.70-2.15	1	N=15			1.65	Very stiff brown slightly sa CLAY. Gravel is fine to m sub-rounded chalk and si	edium sub-angular to	/ × × ×	
2.15	B 8				2.15	Brown silty fine SAND.		/ · · · · · · · · · · · · · · · · · · ·	
3.00 3.00-3.50	D 10	(35)				Stiff orange brown slightly gravelly CLAY. Gravel is sub-angular to sub-round	fine to coarse		
3.50-3.95 4.00-4.50	U 12	(50)				Stiff and very stiff dark gr gravelly CLAY. Gravel is to sub-rounded chalk, qua flint.	fine to coarse angular		
4.50-4.75 4.80 4.80-5.50	D 16	(50)				occasional angular flii 4.80m	nt cobbles below		
5.50-5.95		(50)			5.50	Stiff red boulder CLAY (d driller's daily log)	escription taken from	, , , , , , , , , , , , , , , , , , ,	
6.00 6.00-7.00	D 19 B 20				6.00	Very soft fine sandy sligh Gravel is fine to coarse a siltstone and quartz.			
7.00 7.00-7.30 7.30-8.00		50/170mm			7.00	Stiff orange brown slightly gravelly CLAY. Gravel is to sub-rounded quartz silt	fine to coarse angular		
					8.00	End of Borehole at 8.00 r	m		
		Water Obarter pit to		tions					

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

NICHOLI Tel: 0116		OLTON AI 36333	ND PAI	RTNERS	S LTD.	BOREHOLI (Cable Pe		Bore Num	
Site Hallam	Fiel	ds, Birsta	all. Le	icester	shire.			BI	1 2
Client		<u>,</u>	,			Boring diameter:	Casing diameter:	Logged I	bv: RF
Jelson	Limi	ted.				130 11111 10 6.00111		Scale:	1:50
LR:G040			nd Lev	el:		Date: 22/09/2003	Location: -	Sheet	
		Tests	Water	Level	Depth	Descrip	otion	Legend	Install-
Depth (m)	Туре	SPT N		(mAOD)	(m)	TOPSOIL.			ation
0.20-0.45 0.45-1.15					0.20 0.45	Very stiff slightly sandy C gravel and rootlets. Grave angular to sub-angular fli	el is fine to coarse /		
1.20-1.60 1.65 1.70-1.95	D 4	(18)				Stiff grey and brown sligh Gravel is fine to coarse si sub-rounded chalk, siltsto	ub-angular to		
2.00-2.45 2.50 2.50-2.95	U 6	(25)			2.00	Stiff dark grey slightly gra fine to coarse angular to siltstone, flint and mudsto	sub-rounded chalk,		
3.00-3.35	U 9	(32)							
3.40 3.45-4.00	D 10 D 11								
4.00-4.45	U 12	(35)							
4.50 4.50-5.00	D 13 D 14								
5.00-5.45	U 15	(36)							
5.50 5.50-6.50	D 16 D 17					becoming very stiff be	elow 5.50m.		
6.50-6.95	U 18	(50)							
7.00 7.00-8.00	D 19 D 20								
					8.00	End of Borehole at 8.00 r	n		
		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\							<u> </u>
Kemarks	ano	Water Ob	serva	uons					

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

Tel: 0116 - 2		ND PAI	RTNERS	S LTD.	(Cable Pe	E RECORD rcussion)	Bore Num	
Site							RI	H3
Hallam Fie	elds, Birsta	all, Le	icester	shire.	Devise diemeter	Casing diameters		13
Client					Boring diameter: 150 mm to 8.00m	Casing diameter:	Logged	by: RF
Jelson Lim	ited.						Scale:	1:50
LR:G04003		nd Lev	el:		Date: 19/09/2003	Location: -	Sheet	1 of 1
Samples Depth (m) Typ		Water	Level (mAOD)	Depth (m)	Descrip	tion	Legend	Install- ation
0.00-0.25 D 1			(IIIAOD)		TOPSOIL.			
0.25-1.15 D 2				0.25	Very stiff grey and brown Gravel is fine to coarse ar flint, chalk and siltstone.			
1.55 D 4 1.60-1.95 B 5								
2.00-2.35 U 6								
2.40 D 7 2.45-2.95 D 8								
3.00-3.35 U 9								
3.40 D 1 3.45-3.95 D 1					becoming dark grey b	elow 3.40m		
4.00-4.45 U 1	2 (50)							
4.50 D 1				4.50	Very stiff brown slightly gr is fine to coarse sub-angu			
5.00-5.45 U 1	5 (50)				siltstone and mudstone a sandstone cobble w	ras encountered		
5.50-6.50 D 1	6				between 4.50m and 5.00r	n		
6.50-6.85 U 1	(3.5)							
6.90 D 1 6.95-8.00 D 1				8.00	gravel becoming occa			
Remarks an	d Water Ob	oservat	iions		End of Borehole at 8.00 n	n		

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

NICHOLI Tel: 0116		OLTON AI 36333	ND PAI	RTNERS	S LTD.		E RECORD ercussion)	Bore Num	
Site								DI	H4
Hallam	Fiel	ds, Birsta	all, Le	icester	shire.	Devise a dispersion	Casina diamatan	DI	14
Client						Boring diameter: 150 mm to 8.00m	Casing diameter:	Logged	by: RF
Jelson	Limi	ted.						Scale:	1:50
LR:G040			nd Lev	el:		Date: 22/09/2003	Location: -	Sheet	1 of 1
Samp Depth (m)	oles & Type	Tests SPT N	Water	Level (mAOD)	Depth (m)	Descri	ption	Legend	Install- ation
0.00-0.20 0.20-1.15	D 1	SFIN		(IIIAOD)	0.20	Mixed hard brown slightly TOPSOIL.	y gravelly CLAY and		
1.20 1.30-1.45		(50)			1.45	Very stiff red brown and CLAY. Gravel is fine to c sub-rounded chalk, siltst	oarse anglar to one, quartz and flint.		
1.50-1.85 1.85-2.00 2.00-2.40	D 5 D 6 U 7	(50)			1.85	Orange brown silty fine S		* * * * * * * * * * * * * * * * * * *	
2.45 2.50-3.00	D 8 D 9	` ,			2.40	Stiff red brown slightly gr occasional partings of sil to medium sub-angular to and mudstone.	ty sand. Gravel is fine		
3.50 3.50-4.00 4.00-4.45 4.50 4.50-5.00 5.00-5.30	D 11 D 12 U 13 D 14 D 15 U 16	(35) (42) (50)			5.30	Very stiff brown slightly g is fine to coarse sub-ang siltstone, chalk, quartz ar	ular to sub-rounded nd mudstone.		
5.35 5.40-6.50 6.50 6.50-6.95 7.00-8.00	S D 19 D 20	N=37			8.00	Orange brown silty fine S partings of soft brown cla of weak orange brown si	ay and some fragments Itstone.		
Remarks	s and	Water Ob	oservat	ions		End of doferiole at 8.00			

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

NICHOLI Tel: 0116		OLTON AN 36333	ND PAI	RTNERS	S LTD.	BOREHOLI (Cable Pe	E RECORD rcussion)	Borel Num	
Site Hallam	Fiel	ds, Birsta	all Le	icester	shire				1 5
Client		40, 2	, <u>_</u>	1000101	0101	Boring diameter:	Casing diameter:	Logged k	ov: RF
Jelson I	Limi	ted.				150 mm to 8.00m		Scale:	1:50
LR:G040			nd Lev	el.		Date: 18/09/2003	Location: -	Sheet	
Samp	oles &	Tests	Water	Level	Depth	Descrip		Legend	Install-
Depth (m)	Туре	SPT N		(mAOD)	(m)	TOPSOIL.		\(\lambda\)	ation
0.25-1.15					0.25	Loose orange slightly silty fine SAND. Gravel is fine to sub-rounded siltstone a	to coarse sub-angular		
1.20 1.20-1.65	S D3	N=6							
1.70-1.95 2.00 2.00-2.45	B4 S D6	N=9							
2.50-2.95	В7							******* ******	
3.00 3.00-3.45	S D9	N=8							
3.50-4.00 4.00 4.00-4.45	S	N=14				becoming medium de	ense below 4.00m		
4.50-5.00 5.00	B 13	N=26				g			
5.00-5.45					5.30			×××××	
5.50-6.50	D 16				0.00	Stiff friable red brown slig many partings of grey silt siltstone lithorelicts.	htly sandy CLAY with and occasional		
6.50-6.95	U 17	(35)							
7.00 7.00-8.00	D 18 D 19					becoming very stiff be occasional siltstone lithor	elow 7.00m with elicts		
					8.00	End of Borehole at 8.00 r	n		

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

NICHOLL Tel: 0116			ND PA	RTNERS	S LTD.	BOREHOLI (Cable Pe		Bore Num	
Site								Ы	H6
Hallam	Field	ds, Birst	all, Le	icester	shire.	Boring diameter:	Casing diameter:	DI	10
Client						150 mm to 8.00m	150 mm to 6.00m	Logged I	by: RF
Jelson I	_imit	ed.						Scale:	1:50
LR:G040			ınd Lev	el:		Date: 19/09/2003	Location: -	Sheet	1 of 1
	les & Type	Tests SPT N	Water	Level (mAOD)	Depth (m)	Descrip	otion	Legend	Install- ation
	D 1	OI I IV		(III/(OB)	0.20	TOPSOIL.			
1.20-1.65		(10)			0.20	Very stiff grey and brown Gravel is fine to coarse a chalk siltstone and flint.			
1.70 1.70 1.70-1.95 2.00-2.45	U 2 D 3 D 4 U 5	(19)				becoming stiff below	1.70m		
2.50	D 6				0.70				
2.70-3.00 3.00-3.45	D 7	(20)			2.70	Firm and stiff grey brown	CLAY		
3.50 3.60-3.95 4.00-4.45		(35)			3.60	Stiff brown slightly gravel fine to coarse sub-angula siltstone, chalk, flint and i	r to sub-rounded		
4.50-5.00 5.00 5.00 5.00-5.45	D 13 S D 14 D 15	N=35				becoming very stiff be	elow 4.50m		
5.80-6.45 6.50-6.95		(50)			5.80	Very stiff red brown slight Gravel is fine to coarse a siltstone and mudstone.	tly gravelly CLAY. ngular to sub-rounded		
7.00 7.00-8.00	D 18 D 19				8.00				
					0.00	End of Borehole at 8.00 r	n		

- 1.Hand dug starter pit to 1.20m.
 2.Groundwater seepage observed from 4.50m, rose to 4.30m after 20 minutes, sealed with casing.
 3.Water monitoring well installed to 8.00m.

NICHOLI Tel: 0116		OLTON AI 36333	ND PAI	RTNERS	S LTD.		E RECORD ercussion)	Bore Num	
Site	-	. D: .						RI	H7
	Fiel	ds, Birsta	all, Le	icester	snire.	Boring diameter:	Casing diameter:		
Client						150 mm to 8.00m	Guarria diameter.	Logged	by: RF
Jelson	Limi	ted.						Scale:	1:50
LR:G040			nd Lev	el:		Date: 18/09/2003	Location: -	Sheet	1 of 1
	ples & Type	Tests SPT N	Water	Level (mAOD)	Depth (m)	Descri	ption	Legend	Install- ation
0.00-0.20	D 1	01111		(11,,102)	0.20	TOPSOIL.			
0.20-1.15	D 2				0.20	Brown slightly silty slightl Gravel is fine to coarse a flint quartz and siltstone.	angular to sub-rounded		
1.20 1.20-1.65	S D4	N=8			1.20	Firm and stiff orange bro gravelly CLAY. Gravel is	fine to coarse angular		
1.70-1.95 2.00-2.45		(9)				to sub-rounded flint and	SIITSTONE.		
2.50 2.50-2.95	D 7 D 8				2.50	Soft brown slightly sandy Gravel is fine to coarse a			
3.00-3.45	U 9	(14)				flint, siltstone, quartz and			
3.50 3.50-4.00									
4.00-4.45		(22)				becoming firm below	4.00m		
4.50 4.50-5.00		()			4.50	Soft brown CLAY with oc angular to sub-rounded of			
5.00-5.35 5.40	D 16	(25)			5.40	gravel.			
5.45-6.45	D 17					Firm brown CLAY with or sub-angular to sub-round flint gravel.	ccasional fine to medium ded chalk siltstone and		
6.50-6.95	U 18	(26)							
7.00 7.00-8.00	D 19 D 20					becoming soft below	7.00m		
					8.00	End of Borehole at 8.00 i	m		
Domesil	. 6:	Motor Of	200:::::	tions					
nemarks	ano	Water Ob	, serva	แบบร					

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

NICHOLI Tel: 0116		OLTON AI 36333	ND PA	RTNERS	S LTD.		E RECORD ercussion)	Bore Num	
Site Hallam	Fiel	ds, Birsta	all. Le	icester	shire.			ВІ	H8
Client	1 101	<u>ao, Birot</u>	un, 20	1000101	0111101	Boring diameter:	Casing diameter:	Logged	bv: RF
Jelson	Limi	ted.				150 mm to 8.00m		Scale:	1:50
LR:G040			nd Lev	el:		Date: 17/09/2003	Location: -	Sheet	
Sam	oles &	Tests	Water	Level	Depth	Descri		Legend	Install-
Depth (m) 0.00-0.60	Type D 1	SPT N		(mAOD)	(m)	TOPSOIL.			ation
0.60-1.15 1.20 1.20-1.65	S	N=8			0.60	Loose orange red brown	silty fine SAND		
1.70-2.00	D 5							× × × ×	
2.00 2.00-2.45	S D7	N=11				becoming medium de	ense below 2.00m	× × × × × × × × × × × × × × ×	
2.50-3.00	B 8							X X X X X X X X X X X X X X X X X X X	
3.00 3.00-3.45	S D 10	N=9				becoming loose belo	w 3.00m	* * * * * * * * * * * * * * * * * * *	
3.50-4.00	B 11	N=16						*	
4.00-4.45 4.50-5.00	D 13	IV=10				becoming medium de	ense below 4.00m	X X X X X X X X X X X X X X X X X X X	
5.00-5.45								* * * * * * * * * * * * * * * * * * *	
5.50-6.30	D 17							× × × × × × × × × ×	
6.30-6.50					6.30			X	
6.50 6.50-6.95	s	N=26			6.30	Stiff friable red brown sli many grey green silt and	ghtly sandy CLAY with I fine sand partings		
7.00-8.00	D 21					with occasional band 7.00m.	ls of siltstone below		
					8.00	End of Borehole at 8.00	m		
Remarks	and	Water Ob	oserva	tions					

- Hand dug starter pit to 1.20m.
 No groundwater seepages were encountered during boring operations.
 Water monitoring well installed to 8.00m.

APPENDIX B

TRIAL PIT LOGS

LR: G04003

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Fields	s, Birstall	l, Leic	estersh	ire.			TF	7 1
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level			Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
		itu Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m) 0.50-0.60	Type	Result		(m)	0.30 0.50		rse angular to sub-rounded		ation
						CLAY with some cobb and pockets of fine sai	e gravel. nd grey slightly gravelly bles and occasional boulders nd. Gravel is fine to coarse d flint, chalk and siltstone.		
1.20-1.30	D 2								
2.60-2.70	D3					becoming dark gre	ey and brown below 2.00m.		
					3.00	End of Trial Pit 3.00 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 11%.
 3.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test.
 4.No groundwater seepages were observed.

- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			11	2
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.			ation
0.50-0.60	B 1				0.30	Medium dense* brown occasional fine to coar flint, chalk and siltston	rse angular to sub-rounded		
1.00-1.10	D 2				0.90	CLAY with some cobb and occasional boulde	nd grey slightly gravelly bles, pockets of fine sand ers. Gravel is fine to coarse d flint, chalk and siltstone.		
					1.00	gravelly fine to coarse	s slightly clayey slightly SAND with some cobbles s fine to coarse angular to k and siltstone.		
2.60	D 3				2.60	End of Trial Pit 2.60 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test.
- 4. Trial pit terminated because large siltstone boulder which could not be penetrated was encountered at 2.60m.
- 5.No groundwater seepages were observed.
- 6.Trial pit walls did not collapse.
- 7.Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	nire.	<u> </u>		11	9 3
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
		itu Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.		3000000	ation
0.10-0.20	J 1					TOI SOIL.			
					0.25	Stiff brown CLAY.			
					0.40		y gravally CLAV with		
0.50-0.60	J 2 B 3					Very stiff brown slightly occasional cobbles. Gangular to sub-rounder	ravel is fine to coarse d flint, chalk and siltstone.		
1.00 1.00-1.10	HSV D 4	150 kPa			1.00	CLAY with occasional	d brown slightly gravelly boulders. Gravel is fine to rounded flint, chalk and		
2.00-2.10	D 5				1.80	gravelly CLAY with occ	d greenish grey slightly casional cobbles. Gravel is to sub-rounded flint, chalk		
2.80-2.90	D 6				0.00	occasional pocket 2.50m.	s of gypsum crystals below		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 2.Set of three hand shear vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa. 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Pit Number
Site								
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			TP 4
Client						Method of excavation	Dimensions	Logged by: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25
LR:G040		Ground	Level			Date: 07/10/2003	Location: 0E 0N	Sheet 1 of 1
Sample Depth (m)	Type	situ Tests	Water	Level (m)	Depth (m)	Des	scription	Legend Install- ation
Depth (m)	Туре	Result		(III)	0.30	TOPSOIL.		ation
0.50-0.60	B 1				0.80	Stiff brown slightly grate to coarse sub-angular flint.	velly CLAY. Gravel is fine to sub-rounded chalk and	
1.00 1.10-1.20	HSV D 2	127 kPa			0.80	with occasional cobble	rey slightly gravelly CLAY es. Gravel is fine to coarse d flint, chalk and siltstone.	
1.60-1.70	D3				1.70	from 1.30 to 2.30m .	pocket of gravelly sand	
			•			Very stiff blue grey and gravelly CLAY. Gravel sub-rounded flint, chal	d greenish grey slightly is fine to coarse angular to k and siltstone.	
2.80-2.90	D 4				3.10	End of Trial Pit 3.10 m		

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%.
 3.Set of three Hand Shear Vane (HSV) tests at 1.10m gave average apparent undrained shear strength of 127kPa.
 4.Slight groundwater seepage encountered at 2.30m, (base of pocket of sand) with some sand 'running' into the pit.
- 5. Trial pit walls did not collapse in the clay but was slight collapse in the pocket of sand.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	.TD.	TRIAL PIT RECORD		Trial Num	
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	nire.				5
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.	adv oligibativ grovotly CLAV		ation
0.50-0.60	B 1				1.00	Gravel is fine to coarse flint, chalk and siltston			
1.10	HSV	123 kPa			2.30	Firm and stiff blue grey gravelly CLAY with so brown sand and occas to coarse angular to su siltstone.			
2.70-2.80	D 3				3.00	some cobbles and occ	ghtly gravelly CLAY with casional boulders. Gravel is to sub-rounded flint, chalk		
					5.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 12%. 2.Set of three Hand Shear Vane (HSV) tests at 1.10m gave average apparent undrained shear strength of 123kPa. 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.				96
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson L	imite	d.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
		itu Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.			ation
0.50-0.60	B 1				0.60		velly CLAY. Gravel is fine to sub-rounded chalk and		
1.00 1.00-1.10	HSV D 2	136 kPa			1.50	pockets of orange brow	with occasional cobbles and wn fine to medium sand. e angular to sub-rounded		
1.60-1.70	D 3				1.50	with occasional cobble	wn slightly gravelly CLAY es, boulders and pockets of rel is fine to coarse angular halk and siltstone.		
2.90-3.00	D 4				3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 8%.2.Land drain encountered at 0.80m below ground level running in an East- West direction.3.Set of three Hand Shear Vane (HSV) tests at 1.10m gave average apparent undrained shear strength of 136kPa.
- 4.No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	ire.				7
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson l	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.25	TOPSOIL.			ation
0.50.0.00	D 1				0.50	Firm brown CLAY.			
1.00 1.00-1.10	HSV D 2	150 kPa			1.30	with occasional cobble brown fine sand. Grav to sub-rounded flint, ch	ey and brown below 0.90m.		
						CLAY with occasional coarse angular to subsiltstone.	d brown slightly gravelly cobbles. Gravel is fine to rounded flint, chalk and		
2.80-2.90	D3				3.00	with occasional bould			
						End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.	<u> </u>			9 8
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample	s & In s	situ Tests	Water	Level	Depth		scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TODCOIL and alovemin		30/80/80	ation
0.50-0.60	B 1	150 kPa			0.10	TOPSOIL and clay mix Very stiff brown and groccasional cobbles. Gangular to sub-rounde	rey gravelly CLAY with		
1.30-1.40 2.00-2.10	D 2				1.20	with some cobbles. Gr	wn slightly gravelly CLAY ravel is fine to coarse d flint, chalk and siltstone.		
2.90-3.00	D 4				3.00	Very stiff dark grey slig some cobbles. Gravel sub-rounded flint, chal End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Pit Number
Site								TP 9
	Field	s, Birstall	, Leic	estersh	ire.			173
Client						Method of excavation	Dimensions	Logged by: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25
LR:G040		Ground	Level	•		Date: 07/10/2003	Location: 0E 0N	Sheet 1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend Install- ation
Depth (m)	туре	Result		(111)	0.25	TOPSOIL.		allon
0.50-0.60	D 1						n slightly gravelly CLAY. e angular to sub-rounded e.	
1.00-1.10	D 2					no fissures below	0.80m	
					1.90	fine to medium sand b		
2.00-2.10	D 3				3.00	to coarse SAND.	rown slightly gravelly fine	
						End of Trial Pit 3.00 m		

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 2.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test. 3.No groundwater seepages were observed. 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	'NERS L'	TD.	TRIAL PIT I	RECORD	Trial F Numb	
Site								TD1	10
Hallam	Field	s, Birstall	, Leic	estersh	ire.	Γ	I	TP1	IU
Client						Method of excavation	Dimensions	Logged by:	JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:	:25
LR:G040		Ground	Level	•		Date: 01/10/2003	Location: 0E 0N	Sheet 1	of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription		nstall- tion
0.10-0.20	J 1	Hesuit		(111)		TOPSOIL.			uon
0.40-0.50 0.50-0.60	J2 B3				1.90	flint, chalk and siltston	rse angular to sub-rounded e gravel.		
2.10-2.20	D 4					Firm red brown occasi occasional fine to coar flint, chalk and siltston becoming stiff below			
					2.80	Soft red brown sandy	CLAY.		
3.20-3.30	D 5				3.40	SAND.	e brown fine to medium		
						End of Trial Pit 3.40 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.No groundwater seepages were observed.

- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Pit Number		
Site									144	
Hallam	Field	s, Birstal	I, Leic	estersh	nire.				211	
Client						Method of excavation	Dimensions	Logged	by: JP	
Jelson I	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25	
LR:G040	03	Ground	Level	•		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1	
		situ Tests	Water	Level (m)	Depth (m)	Des	scription	Legend	Install-	
Depth (m)	Туре	Result		(111)		TOPSOIL.			ation	
0.50-0.60	B 1				0.30	gravelly fine to mediun	e brown slightly silty slightly n SAND. Gravel is fine to rounded quartz and flint.			
1.10-1.20 2.60-2.70	D2		Medium dense* red brown slightly clayey si silty fine SAND.				own slightly clayey slightly			
					3.00	End of Trial Pit 3.00 m				

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.No groundwater seepages were observed.

- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLLS Tel: 0116 -) PART	'NERS L'	TD.	TRIAL PIT I	RECORD	Trial Num	
Site					_				
Hallam F	ields	, Birstall	, Leic	estersh	ire.			IP	12
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson Li	mite	d.				Mechanical Excavator	0.70m x 4.00m	Scale:	1:25
LR:G0400	3	Ground	Level:			Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
Samples	& In si	tu Tests	Water	Level	Depth		scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.	un locally sandy CLAV with		ation
	B 1						vn locally sandy CLAY with rse angular to sub-rounded e gravel.		
1.50-1.60	D 3				1.30 1.40 1.60	fine to coarse angular and siltstone.			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 2.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test.
- 3. Could not penetrate deeper than 1.60m below ground level, extended the pit to 4.00m long but still could not penetrate- Trial Pit terminated.
- 4. No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			12	13
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample	s & In s	situ Tests	Water	Level	Depth		scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.			ation
0.50-0.60	B 1				0.30	Very stiff light brown si Gravel is fine to coarse flint, chalk and siltston	e angular to sub-rounded		
					1.30		y gravelly CLAY. Gravel is to sub-rounded flint, chalk		
2.00-2.10	D3				1.90	Very stiff fissured red to CLAY with occasional	prown slightly gravelly cobbles.		
					2.50	End of Trial Pit 2.50 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.2.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test.3.Digging was very slow and finally terminated at 2.50m below ground level when the mechanical excavator could not penetrate any further.
- 4. No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site								TF	1
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			12	P14
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground	Level			Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	Type	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
Deptir (iii)	Туре	Hesuit		(111)	0.30	TOPSOIL.			alion
0.50-0.60						some pockets of brow	lightly gravelly CLAY with n fine to medium sand. e angular to sub-rounded e.		
1.50-1.60	В2				1.20	gravelly fine to mediun	slightly clayey slightly n SAND locally graded to onal lumps of firm brown coarse angular to		
2.70-2.80	D3				2.20		ghtly gravelly CLAY with ravel is fine to coarse d flint, chalk and siltstone.		
					3.00	End of Trial Pit 3.00 m		<u> </u>	

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%.
 3.No groundwater seepages were observed.
 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	ire.			IP	15
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
		itu Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.			ation
0.50-0.60	B 1				0.60		y gravelly CLAY. Gravel is to sub-rounded flint, chalk		
1.00 1.00-1.10	HSV D 2	150 kPa			1.40	with occasional cobble brown fine to medium	wn slightly gravelly CLAY es and pockets of orange sand. Gravel is fine to rounded flint, chalk and		
2.90-3.00	D3				3.00	CLAY with occasional coarse angular to subsiltstone.	d brown slightly gravelly cobbles. Gravel is fine to rounded flint, chalk and		
						End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa.
 3.Land drain encountered at 1.00m below ground level running in a North- South direction.
- 4.No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site								TO	146
Hallam	Field	s, Birstal	l, Leic	estersh	nire.	I	T		16
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 07/10/1930	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	0.30	TOPSOIL.			ation
0.50-0.60	B 1					Stiff light brown slightly gravelly CLAY. Gravel sub-rounded flint, chal	is fine to coarse angular to		
1.10-1.20	D 2 1.10		2.30	gravelly slightly cobble occasional boulders. C angular to sub-rounde	d flint, chalk and siltstone.				
2.70-2.80	D3				3.00	fine to coarse angular and siltstone.	gravelly CLAY. Gravel is to sub-rounded flint, chalk		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 11%. 2.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test. 3.No groundwater seepages were observed. 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Pit Number
Site								TP17
Hallam	Field	s, Birstal	l, Leic	estersh	nire.	T	T	1611
Client						Method of excavation	Dimensions	Logged by: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25
LR:G040	03	Ground	Level	:		Date: 07/10/2003	Location: 0E 0N	Sheet 1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend Install-
Depth (m) 0.50-0.60	Type B 1	Result		(m)	(m) 0.30	cobbles and pockets o medium sand. Gravel sub-rounded flint, chal	is fine to coarse angular to	ation
1.20 1.20-1.30	HSV D 2	123 kPa						
2.60-2.70	D3				2.00 2.10	fine to coarse angular and siltstone. Medium dense* slightly slightly cobbley fine to occasional boulders. Cangular to sub-rounders.	Gravel is fine to coarse d flint, chalk and siltstone.	
						End of Trial Pit 3.00 m		

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 8.7%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.20m gave average apparent undrained shear strength of 123kPa.
 3.*- Denotes relative density assessed visually from the stability of the pit walls, and ease of excavation.
 4.No groundwater seepages were observed.

- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	TNERS L	.TD.	TRIAL PIT I	RECORD	Trial Num	
Site Hallam	Field	s, Birstall	I, Leic	estersh	nire.				18
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
0.50-0.60 0.90 1.00-1.10	B 1 HSV D 2	150 kPa			0.30 0.50	cobbles. Gravel is fine sub-rounded flint, chal Stiff and very stiff light gravelly CLAY with so coarse angular to subsiltstone. Stiff and very stiff dark gravelly CLAY with so boulders and pockets medium sand. Gravel sub-rounded flint, chal	brown and grey slightly me cobbles. Gravel is fine to rounded flint, chalk and grey and brown slightly me cobbles and occasional of orange brown fine to is fine to coarse angular to		
2.80-2.90	D 3				3.00	cobbles. Gravel is fine sub-rounded flint, chal	k and siltstone.		

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 9.0%.
 2.Set of three Hand Shear Vane (HSV) tests at 0.90m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site									
Hallam	Field	s, Birstall	I, Leic	estersh	ire.			IP	19
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample	s & In s	situ Tests	Water	Level	Depth		scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.			ation
0.50-0.60	B 1								
1.00 1.00-1.10	HSV D 2	150 kPa				becoming very stif	ff below 0.90m.		
2.80-2.90	D3				3.00	becoming dark gre occasional cobbles 1.	40m below.		
						End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 9.0%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	ire.			IP	20
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.	un finaurad aliahtlu	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ation
0.50-0.60	B 1				0.90	Stiff and very stiff brow gravelly CLAY. Gravel sub- rounded flint, cha	is fine to coarse angular to		
1.00 1.00-1.10	HSV D 2	139 kPa			2.10	and pockets of orange Gravel is fine to coarse flint, chal and siltstone	casional cobbles, boulders brown fine to medium sand. e angular to sub- rounded		
2.50-2.60	DЗ				3.00	Stiff dark grey and gre gravelly CLAY with occ pockets of grey slightly sand. Gravel is fine to rounded flint, chalk an	casional cobbles and y gravelly fine to coarse coarse angular to sub-		
					3.50	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 7%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of 139kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit iber
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	nire.	<u> </u>		IP	P21
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	l Level	:		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.25	TOPSOIL.			ation
0.50-0.60	B 1						velly CLAY. Gravel is fine ub-rounded flint, chalk and		
1.00 1.00-1.10	HSV D 2	148 kPa			0.70	Stiff and very stiff brow gravelly CLAY with oc- and pockets of orange	vn and grey slightly casional cobbles, boulders brown fine to medium sand.		
					1.60				
2.20-2.30	D 3				1.00	Stiff and very stiff dark gravelly CLAY with och boulders.	grey and brown slightly casional cobbles and		
2.20-2.30	<i>D</i> 3				3.00				
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 8.0%. 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of 148kPa. 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site									22
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			17	
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.	wally CLAY Croyal is fine		ation
0.50-0.60	B 1				0.60		velly CLAY. Gravel is fine ub-rounded flint, chalk and		
1.00-1.10	D 2				1.60	slightly gravelly fine to fine to coarse angular and siltstone.	tly gravelly CLAY and brown coarse SAND. Gravel is to sub-rounded flint, chalk		
	1.6				Stiff brown and grey sl Gravel is fine to coarse flint, chalk and siltston	e angular to sub-rounded			
2.50-2.60	D3				3.00	with occasional cobble			
					3.50	End of Trial Pit 3.00 m			

- Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%.
 No groundwater seepages were observed.
 Trial pit walls did not collapse.
 Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			IP	23
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.		X//XX//XX/	ation
0.10-0.20	J 1					TOPSOIL.			
0.50-0.60	J2 B3				0.30	Very stiff light brown s Gravel is fine to coarse flint, chalk and siltston	e angular to sub-rounded		
							rey slightly gravelly CLAY. e angular to sub-rounded e.		
1.00 1.00-1.10	HSV D 4	127 kPa							
					1.00				
2.20-2.30	D 5				1.90	CLAY with occasional	d brown slightly gravelly cobbles. Gravel is fine to rounded flint, chalk and		
					3.00				
						End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%. 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of 127kPa. 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site								TD	24
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			TP	2 4
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground	Level			Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
		riodal		(w)	0.30	TOPSOIL. Very stiff brown slightly occasional cobbles. Gi	y gravelly CLAY with		anon
0.50-0.60 1.00	B 1	150 kPa					d flint, chalk and siltstone.		
1.20-1.30	D 2				1.50	becoming brown a	and grey below 1.10m		
2.00-2.10	D3				1.50	Very stiff red brown an CLAY. Gravel is fine to sub-rounded siltstone.	d grey slightly gravelly o coarse angular to		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 11%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.10m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber				
Site													
Hallam	Field	s, Birstal	l, Leic	estersh	ire.			IP	25				
Client						Method of excavation	Dimensions	Logged b	oy: JP				
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25				
LR:G040	03	Ground	Level	•		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1				
Sample	s & In s	situ Tests	Water	Level	Depth		scription	Legend	Install-				
Depth (m)	Туре	Result		(m)	(m)			V//8/V//8/V	ation				
					0.00	TOPSOIL.							
0.50-0.60	B 1				0.30	fine SAND with occasi	n slightly clayey slightly silty onal fine to coarse angular halk and siltstone gravel.						
1.00-1.10	D 2				0.80	Medium dense* brown with occasional fine to sub-rounded flint, chal							
					1.80								
	1.80		1.80							Medium dense* orang	e brown silty fine SAND.		
2.60-2.70	D 3				3.00	End of Trial Pit 3.00 m							
						2.13 S. Thai i ii 0.00 III							

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 14%.
 3.No groundwater seepages were observed.

- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site								TE	26
Hallam	Field	s, Birstall	l, Leic	estersh	ire.	Γ	Г	15	20
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground	Level	•		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
Бери (ш)	Туре	Hesuit		(111)	0.30	TOPSOIL.			alion
0.50-0.60	B 1					Very stiff red brown fis occasional cobbles.	sured CLAY with		
1.10-1.20 2.80-2.90	D2				3.00	occasional fine to coar siltstone and quartz gr			
						End of Trial Pit 3.00 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.No groundwater seepages were observed.

- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

	NICHOLI Tel: 0116		LTON AND) PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Client Method of excavation Dimensions Logged by: JF Jelson Limited. Mechanical Excavator 0.70m x 2.50m Scale: 1:25	Site									
Jelson Limited. Mechanical Excavator 0.70m x 2.50m Scale: 1:25	Hallam	Field	s, Birstal	l, Leic	estersh	nire.			IP	21
	Client						Method of excavation	Dimensions	Logged b	y: JP
	Jelson I	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G04003 Ground Level: Date: 02/10/2003 Location: 0E 0N Sheet 1 of	LR:G040	03	Ground	Level	:		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
Samples & In situ Tests Water Level Depth Description Legend Insta	Sample	s & In	situ Tests		Level					Install-
	Depth (m)	Туре	Result		(m)	(m)	TORCOIL		X//XX//XX	ation
TOPSOIL.							TOPSOIL.			
0.30						0.30				
Very stiff red brown fissured CLAY.							Very stiff red brown fis	sured CLAY.		
0.50-0.60 B 1 0.60 0.60	0.50-0.60	B 1				0.60				
Medium dense* red brown locally clayey SILT with XXXXX occasional partings of firm red brown clay.						0.00	Medium dense* red br	own locally clayey SILT with	<u> </u>	
$0.80-0.90$ D 2 Occasional partings of limit red brown clay. $\begin{array}{c c} \times \times$	0.80-0.90	D 2					occasional partings of	iiiii red brown ciay.	$\times \times $	
$oxed{\times \times \times \times}$									<u> </u>	
$\left[\begin{array}{c c} \overline{x} \times \overline{x} \times \overline{x} \\ \underline{x} \times \overline{x} \times \overline{x} \end{array}\right]$									X X X X X X X X X X X X X X X X X X X	
$oxed{\left\langle \begin{array}{c} \times \times \times \times \times \\ 1 \times \times \times \times \times \end{array} \right\rangle}$									X X X X X X X X X X X X X X X X X X X	
$ar{\mathbb{A}} \times \overline{\mathbb{A}} \times \overline{\mathbb{A}} = \{ 1 \times 1 \times 1 \times 1 \in \mathbb{A} : 1 : 1 \in \mathbb{A} : 1 : \mathbf$									X X X X X X X X X X X X X X X X X X X	
lacksquare									X X X X X X X X X X X X X X X X X X X	
$\left[\begin{array}{c c} x \times x \times x \\ x \times x \times x \end{array}\right]$									X X X X X X X X X X X X X X X X X X X	
$ar{ar{ar{ar{ar{ar{ar{ar{ar{ar{$									X X X X X X X X X X X X X X X X X X X	
lacksquare									XXXX XXXXX	
$ar{\mathbb{A}} = ar{\mathbb{A}} = ar{$									\[\bar{\times} \times \	
lacksquare									X X X X X X X X X X X X X X X X X X X	
$ar{ar{\chi}}_{ar{\chi}} \times \overline{\chi}_{ar{\chi}} = \overline{\chi}_{ar{\chi}} \overline{\chi}_$									X X X X X X X X X X X X X X X X X X X	
racksquare									X X X X X X X X X X X X X X X X X X X	
lacksquare									$1 \times \times \times \times$	
$\begin{bmatrix} 280-290 & D3 \end{bmatrix}$	2.80-2.90	D 3				2.80			$\times \times \times \times \times$	
Medium dense* orange brown fine to medium SAND with occasional fine to coarse angular to							Medium dense* orang	e brown fine to medium SAND		
3.00 with occasional line to coarse angular to sub-angular siltstone gravel.						3.00			1	
End of Trial Pit 3.00 m							End of Trial Pit 3.00 m)		

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.

 2.Set of three Mexi Probe tests undertaken at 0.50m at the Southern end of the pit gave average apparent CBR value of >14% and three taken at the Northern end of the pit gave average apparent CBR value of 6%.
- 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Pit Number
Site								
Hallam	Field	s, Birstall	, Leic	estersh	ire.			TP28
Client						Method of excavation	Dimensions	Logged by: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25
LR:G040		Ground	Level	<u> </u>		Date: 07/10/2003	Location: 0E 0N	Sheet 1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.		ation
0.50-0.60	B 1				0.30	Loose* orange brown a gravelly fine to mediun coarse angular to subsiltstone.	n SAND. Gravel is fine to	
1.20-1.30	D 2							
2.80-2.90	D3				3.00	_	nally gravelly below 2.00m.	
						End of Trial Pit 3.00 m		

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 2%. 3.No groundwater seepages were observed.

- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			IP	29
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson l	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 07/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth		scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.		X//\\/\\	ation
					0.20	TOPSOIL.			
0.50-0.60	B 1				0.50	Medium dense* brown gravelly fine to mediun coarse angular to subsiltstone.	n SAND. Gravel is fine to		
						Medium dense* orang- with occasional fine to sub-rounded quartz ar			
1.20-1.30	D 2								
2.80-2.90	D3				3.00	End of Trial Pit 3.00 m			

^{1.* -} Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.No groundwater seepages were observed.

^{4.} Trial pit walls did not collapse.

^{5.} Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			IP	30
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground	Level			Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
		Hesuit		(11)	0.30	TOPSOIL. Dense* brown silty slig	ghtly gravelly fine to is fine to coarse angular to		alion
0.50-0.60	B 1				0.70	very stiff red brown fis	k and siltstone.		
1.00-1.10	D 2				1.50				
1.60-1.70 2.80-2.90	D3				2.80	Medium dense* red br to medium SAND.	own and light grey silty fine		
2.50-2.30	2 4				3.00	Firm red brown sandy bands of fine to mediu cemented fine to mediu End of Trial Pit 3.00 m	m SAND and pockets of um sand.		

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test.

- 4.No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PAR1	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site								TE	231
Hallam	Field	s, Birstall	l, Leic	estersh	ire.				3 1
Client						Method of excavation	Dimensions	Logged b	by: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground	Level	:		Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
	s & In s	situ Tests	Water	Level (m)	Depth (m)	Des	scription	Legend	Install-
Depth (m)	туре	Result		(III)	0.30	TOPSOIL.	obali o mana alla fina da		ation
0.50-0.60	B 1					Dense* brown silty slig coarse SAND with silts cobbles. Gravel is fine sub-rounded flint, chal	stone and occasional flint to coarse angular to		
					1.10			××××	
					1.20	Very stiff red brown fis	sured CLAY.		-
2.80-2.90	B 2				3.00	Medium dense* orange SAND.	e brown slightly silty medium		

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 14%.
 3.No groundwater seepages were observed.
 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site								TO	20
Hallam	Field	s, Birstal	I, Leic	estersh	nire.	I		IF	232
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
Берит (ті)	Туре	nesuit		(111)	0.30		occasional fine to coarse		alion
0.50-0.60	B 1				0.50	angular to sub-rounder gravel.	d flint, chalk and siltstone		
							velly CLAY. Gravel is fine ub-rounded flint, chalk and		
1.20 1.20-1.30	HSV D 2	150 kPa			1.10	Very stiff red brown sli Gravel is fine to coarse flint, chalk and siltston	e angular to sub-rounded		
2.40-2.50	D3				2.10	Medium dense* red br	own fine SAND.		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 12%. 2.Set of three Hand Shear Vane (HSV) tests at 1.20m gave average apparent undrained shear strength of >150kPa. 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Pit Number	
Site								TD22	,
Hallam	Field	s, Birstall	l, Leic	estersh	ire.		Г	TP33)
Client						Method of excavation	Dimensions	Logged by: JP	
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25	
LR:G040		Ground	Level			Date: 01/10/2003	Location: 0E 0N	Sheet 1 of	1
Sample Depth (m)		situ Tests	Water	Level (m)	Depth (m)	Des	scription	Legend Install ation	l-
Depth (m)	Туре	Result		(III)	0.30	TOPSOIL.	aurad CL AV with many	alion	
0.50-0.60	B 1					rootlets and occasiona	sured CLAY with many Il fine to coarse angular to dstone, siltstone and quartz		
1.10-1.20	D 2				1.30	Stiff red brown slightly occasional pockets of coarse angular to suband sandstone.	gravelly CLAY with silt/clay. Gravel is fine to rounded quartz, siltstone		
2.00-2.10	D 3				2.70				
2.70 2.00	<i>D</i> 4				2.90	Firm and stiff brown C silt. End of Trial Pit 2.90 m	LAY with many partings of	××	
						End of Frial Fit 2.30 III			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 2.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test. 3.No groundwater seepages were observed. 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND) PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site									
Hallam	Field	s, Birstal	I, Leic	estersh	ire.			IP	234
Client						Method of excavation	Dimensions	Logged b	oy: JP
Jelson I	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	:		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.			ation
0.50-0.60	B 1				0.30		occasional fine to coarse d flint, chalk and siltstone		
0.90	HSV	150 kPa				Very stiff brown slightly	y gravelly CLAY. Gravel is to sub-rounded flint, chalk		
2.40-2.50	D 3				1.00	CLAY with occasional	e angular to sub-rounded		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%.
 2.Set of three Hand Shear Vane (HSV) tests at 0.90m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PAR1	NERS L	TD.	TRIAL PIT RECORD		Trial P	
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			TP3	5
Client						Method of excavation	Dimensions	Logged by: J	JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:2	25
LR:G040		Ground	Level	:		Date: 08/10/2003	Location: 0E 0N	Sheet 1 c	of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend Instal	stall-
0.50-0.60	В1	Hesuit		(11)	0.30	to medium sand and o	some pockets of brown fine ccasional gravel. Gravel is		<u>511</u>
1.00-1.10	D 2				0.80	and siltstone. Stiff brown and grey g	ravelly CLAY. Gravel is fine ub-rounded flint, chalk and		
					1.90				
2.00-2.10	D3					with occasional cobble	wn slightly gravelly CLAY es and boulders. Gravel is to sub-rounded flint, chalk		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 10%. 2.Hand Shear Vane tests were aborted because the ground was too gravelly. 3.No groundwater seepages were observed. 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Pit Number	
Site								TP36	
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			1730	
Client						Method of excavation	Dimensions	Logged by: JP)
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25	
LR:G040		Ground	Level			Date: 02/10/2003	Location: 0E 0N	Sheet 1 of	1_
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend Instal	
Беріп (пі)	туре	nesuit		(111)	0.30	TOPSOIL.		allon	
0.50-0.60	B 1				1.00		y gravelly CLAY. Gravel is to sub-rounded flint, chalk		
1.20-1.30	D2				1.00		sand and gravel. Gravel is to sub-rounded chalk with		
2.70-2.80	D3				2.20	CLAY with occasional coarse angular to subsiltstone.	d brown slightly gravelly cobbles. Gravel is fine to rounded flint, chalk and		
					3.50	End of Trial Pit 3.00 m			

- Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 9.0%.
 No groundwater seepages were observed.
 Trial pit walls did not collapse.
 Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site									
Hallam	Field	s, Birstal	l, Leic	estersh	ire.	<u> </u>		IP	237
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m) 0.30	TOPSOIL.			ation
0.50-0.60	В1				0.30	Stiff brown slightly grate to coarse angular to susiltstone.	velly CLAY. Gravel is fine ub-rounded flint, chalk and		
0.90 0.90-1.00	HSV D 2	108 kPa			1.20	Stiff brown and grey sl	ightly gravelly CLAY with		
					2.00	occasional cobbles, bo orange brown fine to n	oulders and pockets of nedium sand. Gravel is fine to rounded flint, chalk and		
2.20-2.30	D3				2.00	Very stiff dark grey and CLAY with occasional pockets of fine brown s	d brown slightly gravelly cobbles, boulders and sand.		
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 11%.
 2.Set of three Hand Shear Vane (HSV) tests at 0.90m gave average apparent undrained shear strength of 108kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site Hallam	Field	s, Birstal	l, Leic	estersh	nire.				38
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 08/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)		situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install-
Беріп (ті)	Туре	Result		(111)	0.30		vn CLAY with occasional		ation
0.50-0.60 0.80-0.90	B 1				0.60	and siltstone gravel.	to sub-rounded flint, chalk velly CLAY with occasional d pockets of sand.		
0.90	HSV	105 kPa			1.90				
2.50-2.60	D3				3.00	CLAY with occasional Gravel is fine to coars flint, chalk and siltston	e angular to sub-rounded e.		
					3.50	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 8.0%. 2.Set of three Hand Shear Vane (HSV) tests at 0.90m gave average apparent undrained shear strength of 105kPa. 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			IP	239
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson l	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.		\/\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	ation
0.10-0.20	J 1					TOPSOIL.			
0.50-0.60	J 2 B 3				0.30	Very stiff brown slightly occasional boulders at fine sand. Gravel is fin sub-rounded flint, chal	nd pockets of orange brown le to coarse angular to		
1.00	HSV	150 kPa				becoming brown a	and grey below 0.80m.		
1.40-1.50	D 4								
2.80-2.90	D 5				3.00				
						End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 7.0%.
 2.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa.
 3.No groundwater seepages were observed.
- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial Num		
Site										
Hallam	Field	s, Birstal	I, Leic	estersh	ire.	I		TP	40	
Client						Method of excavation	Dimensions	Logged b	y: JP	
Jelson I	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25	
LR:G040	03	Ground	Level	:		Date: 02/10/2003	Location: 0E 0N	Sheet	1 of 1	
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-	
Depth (m)	Type	Result		(m)	(m)	TOPSOIL.		ation		
0.50-0.60	B 1	115 kPa			0.35	Gravel is fine to coarse	Very stiff brown and grey slightly gravelly CLAY. Gravel is fine to coarse angular to sub-rounded flint, chalk and siltstone.			
1.30-1.40	D 2	IIJKFA								
	1.90		1.90	with occasional co	obbles below 1.50m. d brown slightly gravelly					
2.80-2.90	D3				3.00	CLAY with occasional coarse angular to subsiltstone.	cobbles. Gravel is fine to rounded flint, chalk and			
						End of Trial Pit 3.00 m				

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 7.0%.
- 2.Land drain encountered at 0.85m below ground level running in an East- West direction.
- 3.Cement asbestos pipe (60mm OD) encountered in southern end of pit at 0.80m below ground level running in a North West-South East direction.
- 4.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of 115kPa.
- 5.No groundwater seepages were observed.
- 6.Trial pit walls did not collapse.
- 7. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD		Trial P Numbe	
Site								TP4	4
	Field	s, Birstall	l, Leic	estersh	ire.			164	• •
Client						Method of excavation	Dimensions	Logged by:	JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:2	25
LR:G040						Date: 01/10/2003	Location: 0E 0N	Sheet 1	of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription		stall- ion
Boptii (iii)	0.30					TOPSOIL.			011
0.50-0.60	B 1				0.30	Very stiff brown slightly fine to coarse angular			
2.00-2.10	D3				1.20	fine to coarse angular	fine to coarse SAND and to sub-rounded flint and ingular siltstone and flint		
2.00-2.10	D3				3.00				
					3.00	End of Trial Pit 3.00 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 14%. 2.Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test. 3.*- Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 4.No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

	NICHOLLS COLTON AND PARTNERS LTD. Tel: 0116 - 2536333					TRIAL PIT I	RECORD	Trial Num	
Site								TP	42
	Field	s, Birstall	, Leic	estersn	ııre.		<u> </u>		
Client						Method of excavation	Dimensions	Logged b	
Jelson L						Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground situ Tests			Τ	Date: 01/10/2003	Location: 0E 0N	Sheet	
Depth (m)	Type	Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
					0.30	TOPSOIL.			
0.50-0.60	B 1					Very stiff brown fissure fine to coarse angular gravel.	ed CLAY with occasional to sub-rounded flint		
					1.10	Very stiff brown slightly fine to coarse angular	y gravelly CLAY. Gravel is to sub-rounded flint.		
1.20-1.30	D 2				1.80		AY with occasional fine to rounded quartz and siltstone		
2.00-2.10	D3				2.40		e brown slightly gravelly fine o coarse sub-angular to and siltstone.		
2.50-2.55	D 4				2.50 2.55 2.55		gravelly CLAY. Gravel is ular to sub-rounded quartz	××××××	
						\Medium strong red bro SILTSTONE. End of Trial Pit 2.55 m			

- 1.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of 12%.
 2. Attempted Hand Shear Vane (HSV) test at 1.00m but the ground was too hard for the test.
 3. *- Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 4.Could not penetrate the siltstone below 2.55m.
- 5. No groundwater seepages were observed.
- 6. Trial pit walls did not collapse.
- 7. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PART	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site									
Hallam	Field	s, Birstall	l, Leic	estersh	ire.			IP	43
Client						Method of excavation	Dimensions	Logged b	y: JP
Jelson L	imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	03	Ground	Level	•		Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
		situ Tests	Water	Level	Depth	Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.			ation
0.50-0.60	B 1				0.30		slightly clayey silty slightly cally cemented. Gravel is to sub-rounded flint.	× × × × × × × × × × × × × × × × × × ×	
					0.80		velly CLAY. Gravel is fine to sub-rounded chalk and		
1.00-1.10	D 2				1.00	Medium dense* brown SAND with some cobb Gravel is fine to coarse quartz, siltstone and fli			
2.00-2.10	D 3				1.90	Medium dense* orang SAND.	e brown fine to medium		
					3.00	End of Trial Pit 3.00 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation. 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%. 3.No groundwater seepages were observed.

- 4. Trial pit walls did not collapse.
- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PAR1	NERS L	TD.	TRIAL PIT RECORD			Pit ber
Site								TE	244
Hallam	Field	s, Birstall	l, Leic	estersh	ire.				44
Client						Method of excavation	Dimensions	Logged I	oy: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040		Ground	Level	:		Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install-
Depth (m)	Туре	Hesuit		(111)		TOPSOIL.			ation
0.50-0.60	B 1				0.30	SAND with occasional	silty slightly gravelly fine partings of clay. Gravel is to sub-rounded flint, chalk		
1.20-1.30	D 2				1.00	Medium dense* orang with occasional fine to sub-rounded quartz gr			
2.50-2.60	D3				3.00	End of Trial Pit 3.00 m			

 ^{1.*-} Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
 3.No groundwater seepages were observed.
 4.Trial pit walls did not collapse.

^{5.} Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PAR1	NERS L	TD.	TRIAL PIT RECORD		Trial Num	
Site								TE)/5
Hallam	Field	s, Birstall	, Leic	estersh	ire.			TP45	
Client						Method of excavation	Dimensions	Logged I	by: JP
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040						Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
Бери (ш)	Туре	riesuit		(111)	0.30	TOPSOIL.			ation
0.50-0.60	B 1					coarse SAND. Gravel	Dense* brown silty slightly gravelly fine to coarse SAND. Gravel is fine to coarse angular to sub-rounded flint, chalk and siltstone.		
1.10-1.20	D 2				0.90	Medium dense* orang to coarse SAND. Grav to sub-rounded flint qu	e brown slightly gravelly fine el is fine to coarse angular artz and siltstone.		
1.90-2.00	D3				1.70	Medium dense* orang coarse SAND. Gravel sub-rounded flint, chal	e brown gravelly fine to is fine to coarse angular to k and siltstone.		
					3.00				. e. u
						End of Trial Pit 3.00 m			

^{1.* -} Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
3.No groundwater seepages were observed.
4.Trial pit walls did not collapse.

^{5.} Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND 6333	PART	NERS L	TD.	TRIAL PIT RECORD			Pit nber
Site								TC	P 46
Hallam	Field	s, Birstall	l, Leic	estersh	ire.		Г		40
Client						Method of excavation	Dimensions	Logged I	by: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040						Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)		situ Tests	Water	Level	Depth (m)	Des	scription	Legend	Install- ation
Бериі (ііі)						TOPSOIL.			alion
0.50-0.60	B 1					is fine to coarse angula chalk and siltstone.	asional flint cobbles. Gravel ar to sub-rounded flint,		
1.40-1.50	D2	2			1.30	Medium dense* red br coarse SAND with occ cobbles. Gravel is fine sub-rounded flint, chal			
2.80-2.90	D3				3.10	End of Trial Pit 3.10 m			

^{1.* -} Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
3.No groundwater seepages were observed.
4.Trial pit walls did not collapse.

^{5.} Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PAR1	NERS L	TD.	TRIAL PIT I	RECORD	Trial Num	
Site								TE	947
Hallam	Field	s, Birstall	l, Leic	estersh	ire.				4/
Client						Method of excavation	Dimensions	Logged I	by: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	04003 Ground Level:					Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
	Samples & In situ Tests Water Level Deptr					Des	scription	Legend	Install-
Depth (m)	Туре	Result		(m)	(m)	TOPSOIL.		X//\X/\X	ation
0.10-0.20 0.40-0.50 0.50-0.60	J 2 B 3				0.20	Medium dense* brown	slightly clayey silty slightly cally cemented. Gravel is to sub-rounded flint.		
1.20-1.30 3.00-3.10	D 4				3.20	to medium SAND with brown clay. Gravel is f sub-rounded flint.	e brown slightly gravelly fine occasional partings of firm ine to coarse angular to		
					3.20	End of Trial Pit 3.20 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
 3.No groundwater seepages were observed.
 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PAR1	TNERS L	TD.	TRIAL PIT RECORD			Pit nber
Site								TE	248
Hallam	Field	s, Birstall	l, Leic	estersh	ire.				40
Client						Method of excavation	Dimensions	Logged I	by: JP
Jelson l	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale:	1:25
LR:G040	'					Date: 01/10/2003	Location: 0E 0N	Sheet	1 of 1
Sample Depth (m)	s & In s	Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
Bopin (iii)						TOPSOIL.			8
0.50-0.60	B 1				0.30		slightly clayey silty slightly ally cemented. Gravel is to sub-rounded flint.		
1.40-1.50	D 2	D 2				to medium SAND with	e brown slightly gravelly fine occasional flint cobbles. e angular to sub-rounded		
2.80-3.00	D3				3.10	End of Trial Pit 3.10 m			

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
 3.No groundwater seepages were observed.
 4.Trial pit walls did not collapse.

- 5. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND) PART	'NERS L'	TD.	TRIAL PIT I	Trial Pi		
Site								TD40	<u></u>
Hallam	Field	s, Birstall	, Leic	estersh	ire.		T	TP49	9
Client						Method of excavation	Dimensions	Logged by: JI	Р
Jelson L	_imite	ed.				Mechanical Excavator	0.70m x 2.50m	Scale: 1:25	5
LR:G040		Ground	Level	•		Date: 01/10/2003	Location: 0E 0N	Sheet 1 o	f 1
Sample Depth (m)	s & In s	situ Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend Insta	
0.00	0	Hesuit		(111)		TOPSOIL.		alion	
0.50-0.60	B 1				0.30		slightly clayey silty slightly cally cemented. Gravel is to sub-rounded flint.		
1.00	HSV	150 kPa			0.90	Very stiff red brown fis occasional fine to coar	sured CLAY with se angular to sub-rounded	0 0 0 0 0	
1.20-1.30	D 2				1.90	flint, quartz and sands	tone gravel.		
2.20-2.30	D3				2.10		e brown slightly gravelly fine vel is fine to coarse angular		
					2.70		CLAY with some pockets of gravelly sand. Gravel is to sub-rounded flint.		
2.80-2.90	D 4				2.70	Soft red brown and light clayey fine to medium	ht grey sandy CLAY / very SAND.	× × × × × × × × × × × × × × × × × × ×	
					3.10	End of Trial Pit 3.10 m	,		

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
 3.Set of three Hand Shear Vane (HSV) tests at 1.00m gave average apparent undrained shear strength of >150kPa.
- 4.No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

NICHOLL Tel: 0116		LTON AND	PAR	TNERS L	TD.	TRIAL PIT I	Trial Num		
Site Hallam	Field	s, Birstall	l. l eid	estersh	ire			ТР	50
Client		c, Dirotan	., _0.0	30.0101	0.	Method of excavation	Dimensions	Logged b	
Jelson I	imite	h.d				Mechanical Excavator	0.70m x 2.50m		1:25
LR:G040		Ground	ا ا میروا			Date: 01/10/2003	Location: 0E 0N	Sheet	
		itu Tests	Water	Level	Depth		scription		Install-
Depth (m)	Туре	Result	· · · · · ·	(m)	(m)			V//AV//AV/	ation
	0.20				0.30	TOPSOIL.			
0.50-0.60	B 1				0.30	Medium dense* light b slightly gravelly fine SA coarse angular to sub-	rown locally clayey silty AND. Gravel is fine to rounded flint.		
1.20 1.30-1.40	HSV 111 kPa 1.10				1.10	Stiff red brown and ligh			
2.60-2.70	D3				2.50	Very weak light grey w grained SANDSTONE	rith some red brown fine		
					3.10	Stiff red brown CLAY.		——————————————————————————————————————	
					3.30	End of Trial Pit 3.30 m		_ =	

- 1.* Denotes relative density assessed visually from the stability of pit walls, and ease of excavation.
 2.Set of three Mexi Probe tests undertaken at 0.50m gave average apparent CBR value of >14%.
 3.Set of three Hand Shear Vane (HSV) tests at 1.20m gave average apparent undrained shear strength of 111kPa.
- 4.No groundwater seepages were observed.
- 5. Trial pit walls did not collapse.
- 6. Trial pit was backfilled with arisings upon completion.

APPENDIX C

SOAKAWAY PIT LOGS

LR: G04003

NICHOLLS COLTON AND PARTNERS LTD. Tel: 0116 - 2536333					TRIAL PIT I	RECORD		Trial Numl	
Site								SK	. 1
Hallam Field	ds, Birstall	l, Leic	estersh	nire.		T		<u> </u>	\ I
Client					Method of excavation	Dimensions	L	ogged by	/: JP
Jelson Limit	ed.				Mechanical Excavator	0.60m x 1.50m	S	Scale:	1:15
LR:G04003	Ground	Level			Date: 22/10/2003	Location: -	5	Sheet 1	1 of 1
Samples & In Depth (m) Type		Water	Level (m)	Depth (m)	Des	scription	L		Install- ation
Sopar (m) Typo	, items		()	0.30	TOPSOIL Brown slightly gravelly	fine to medium SAND			
				2.15	End of Trial Pit 2.15 m	fine to medium SAND.			

Remarks and Water Observations

- No groundwater seepages were observed.
 Trial pit walls did not collapse.
 The pit was backfilled with gravel from 1.00m to 2.15m, monitoring pipe installed to the base.
 The remainder of the pit was backfilled with arisings to ground level.
- 5. See separate sheet for soakaway test results.

NICHOLLS COLTON AND PARTNERS LTD. Tel: 0116 - 2536333			TRIAL PIT I	RECORD	Trial Pit Number					
Site										
Hallam Fields, Birstall, Leicestershire.							Sk	2	.	
Client Method of excavation Dimensions							Logged b	y:		
Jelson L	imite	ed.				Mechanical Excavator	0.60m x 1.70m	Scale:	1:15	
LR:G040	03	Ground	Level:			Date: 22/10/2003	Location: -	Sheet	1 of	1
		situ Tests	Water	Level	Depth (m)	Des	scription	Legend	Insta	
Depth (m)	Type	Result		(m)	0.30	TOPSOIL. Brown slightly gravelly End of Trial Pit 2.20 m			ation	

Remarks and Water Observations

- No groundwater seepages were observed.
 Trial pit walls did not collapse.
 The pit was backfilled with gravel from 1.00m to 2.20m, monitoring pipe installed to the base.
 The remainder of the pit was backfilled with arisings to ground level.
- 5. See separate sheet for soakaway test results.

NICHOLLS COLTON AND PARTNERS LTD. Tel: 0116 - 2536333					.TD.	TRIAL PIT I	RECORD	Trial Num	
Site									
Hallam	Fields	s, Birstall	l, Leic	estersh	nire.	ı		or	(3
Client						Method of excavation	Dimensions	Logged b	y:
Jelson L	imite	d.				Mechanical Excavator	0.60m x 1.40m	Scale:	1:15
LR:G040		Ground	Level	•		Date: 22/10/2003	Location: -	Sheet	1 of 1
Sample Depth (m)	s & In s	itu Tests Result	Water	Level (m)	Depth (m)	Des	scription	Legend	Install- ation
	31-2				0.30	TOPSOIL. Stiff brown slightly san	ndy slightly gravelly CLAY		
					0.70				
					0.70	Very stiff light grey and CLAY	d brown slightly gravelly		
					1.60	Stiff dark grey CLAY			
					2.10	End of Trial Pit 2.10 m			

Remarks and Water Observations

- No groundwater seepages were observed.
 Trial pit walls did not collapse.
 The pit was backfilled with gravel from 1.00m to 2.10m, monitoring pipe installed to the base.
 The remainder of the pit was backfilled with arisings to ground level.
- 5. See separate sheet for soakaway test results.

APPENDIX D

INSITU SOAKAWAY TEST RESULTS

LR: G04003

Page 1 of 2

Trialpit No.: SK 1

Soil Profile:

Depth (m) Description

From: To:

0.00 0.30 Topsoil.

0.30 2.15 Brown slightly gravelly fine to medium SAND.

Sketch plan of test zone

Not to scale

All dimensions in metres.

Test zone filled with pea gravel,

porosity (N) = 0.41

(measured in laboratory)

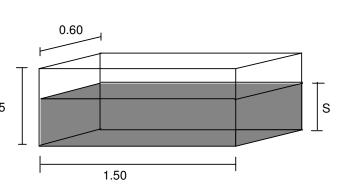
S= Storage depth (m) 2.15

Gravel from 1.00m to 2.15m.



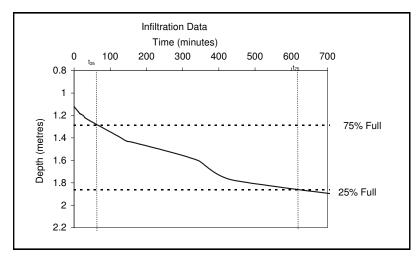
$$a_{p50} = 3.32 \text{ m}^2$$

$$V_{p75-25} = 0.52 \text{ m}^3$$



Soakaway Test Run 1

Test Date: 22/10/2003



Time	Depth
(minutes)	(m)
0	1.12
5	1.14
10	1.16
15	1.18
20	1.19
25	1.20
30	1.22
35	1.23
45	1.25
119	1.38
162	1.44
342	1.60
445	1.78
1333	2.14

From the above graph,

$$t_{p25}$$
= 70 (min) t_{p75} = 620 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.94E-06 $a_{p50} \times t_{p75-25}$

$$f_{\text{run1}} = 1.94 \times 10^{-6}$$
 m/s

Test and analysis carried out in general accordance with BRE Digest 365: 1991

Job No.: G04003

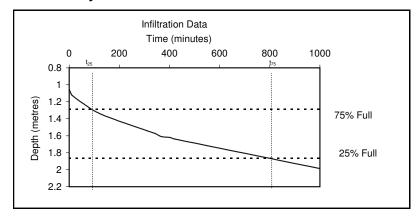
Site: Hallam Fields, Birstall, Leicester.

Client: Jelson Ltd

Page 2 of 2

Trialpit No.: SK 1

Soakaway Test Run 2 Test Date: 23/10/2003



Time	Depth
(minutes)	(m)
0	1.05
8	1.11
10	1.12
14	1.13
51	1.21
130	1.35
310	1.54
341	1.57
368	1.61
402	1.62
424	1.64
1358	2.20

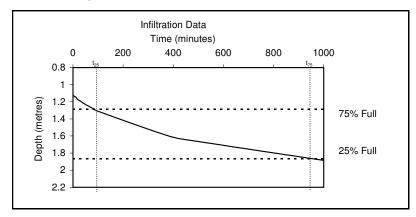
From the above graph,

$$t_{p25}$$
= 80 (min) t_{p75} = 795 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.49E-06
 $a_{p50} \times t_{p75-25}$

$$f_{run2} = 1.49 \times 10^{-6}$$
 m/s

Soakaway Test Run 3 Test Date: 24/10/2003



Time	Depth
(minutes)	(m)
0	1.12
2	1.13
8	1.14
13	1.15
23	1.18
28	1.19
43	1.22
80	1.28
394	1.61
533	1.68
1533	2.12

From the above graph,

$$t_{p25}$$
= 85 (min) t_{p75} = 950 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.23E-06

$$f_{run3} = 1.23 \times 10^{-6}$$
 m/s

Test and analysis carried out in general accordance with BRE Digest 365: 1991

Job No.: G04003

Site: Hallam Fields, Birstall, Leicester.

Client: Jelson Ltd

Page 1 of 2

Trialpit No.: SK 2

Soil Profile:

Depth (m) Description

From: To:

0.00 0.30 Topsoil.

0.30 2.20 Brown slightly gravelly fine to medium SAND.

Sketch plan of test zone

Not to scale

All dimensions in metres.

Test zone filled with pea gravel,

porosity (N) = 0.41

(measured in laboratory)

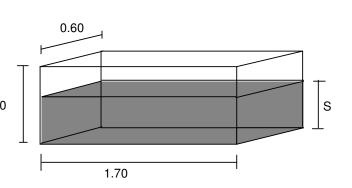
S= Storage depth (m) 2.20

Gravel from 1.00m to 2.20m.



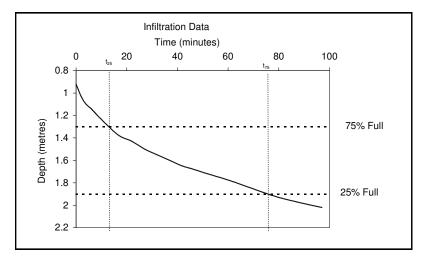
$$a_{p50} = 3.78 \text{ m}^2$$

$$V_{p75-25} = 0.61 \text{ m}^3$$



Soakaway Test Run 1

Test Date: 22/10/2003



Time	Depth
(minutes)	(m)
0	0.92
4	1.10
8	1.10
17	1.38
27	1.28
37	1.28
45	1.67
62	1.50
87	1.50
97	2.02
•	

From the above graph,

$$t_{p25}$$
= 12 (min) t_{p75} = 75 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.76E-05

 $a_{p50} \ x \ t_{p75\text{-}25}$

$$f_{\text{run1}} = 1.76 \times 10^{-5}$$
 m/s

Test and analysis carried out in general accordance with BRE Digest 365: 1991

Job No.: G04003

Site: Hallam Fields, Birstall, Leicester.

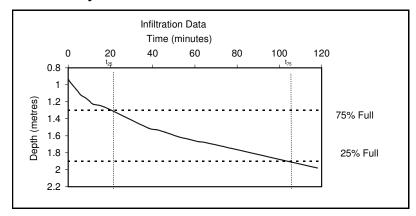
Client: Jelson Ltd

Page 2 of 2

Trialpit No.: SK 2

Soakaway Test Run 2





Time	Depth
(minutes)	(m)
0	0.93
2	1.00
4	1.06
8	1.15
10	1.19
12	1.23
17	1.26
38	1.51
42	1.53
52	1.61
57	1.64
65	1.68
118	1.98

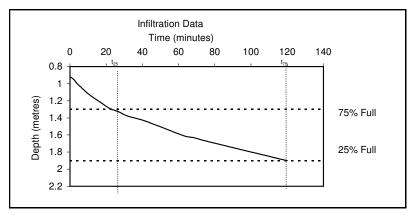
From the above graph,

$$t_{p25}$$
= 22 (min) t_{p75} = 104 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.35E-05
 $a_{p50} \times t_{p75-25}$

$$f_{run2} = 1.35 \times 10^{-5}$$
 m/s

Soakaway Test Run 3 Test Date: 23/10/2003



Time	Depth
(minutes)	(m)
0	0.92
2	0.95
6	1.04
8	1.08
17	1.22
22	1.29
27	1.33
42	1.44
63	1.61
114	1.87
120	1.90

From the above graph,

$$t_{p25}$$
= 23 (min) t_{p75} = 120 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.14E-05

$$f_{run3} = 1.14 \times 10^{-5}$$
 m/s

Test and analysis carried out in general accordance with BRE Digest 365: 1991

Job No.: G04003

Site: Hallam Fields, Birstall, Leicester.

Client: Jelson Ltd

Page 1 of 1

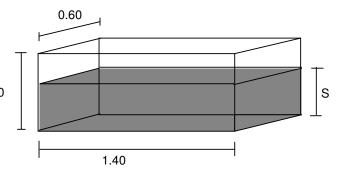
Trialpit No.: SK 3

Soil Profile:

Depth (m)		Description
From:	To:	
0.00	0.30	Topsoil.
0.30	0.70	Stiff brown slightly sandy slightly gravelly CLAY.
0.70	1.60	Very stiff light grey and brown slightly gravelly CLAY.
1.60	2.10	Stiff dark grey CLAY.

Sketch plan of test zone

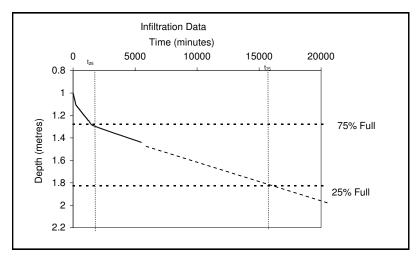
Not to scale
All dimensions in metres.
Test zone filled with pea gravel,
porosity (N) = 0.41
(measured in laboratory)
S= Storage depth (m) 2.10
Gravel from 1.00m to 2.10m.



Gives the Figures

S=	1.10	m
a _{p50} =	3.04	m ²
V _{p75-25} =	0.46	m^3

Soakaway Test Run 1 Test Date: 22/10/2003



Time	Depth
(minutes)	(m)
0	1.00
124	1.06
195	1.09
253	1.11
1181	1.24
1295	1.25
1616	1.29
5502	1.44

From the above graph, (T₇₅ extrapolated)

$$t_{p25}$$
= 1400 (min) t_{p75} = 16000 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 7.11E-08 $f_{run1} = 7.11 \times 10^{-8}$ m/s

Test and analysis carried out in general accordance with BRE Digest 365: 1991

Job No.: G04003

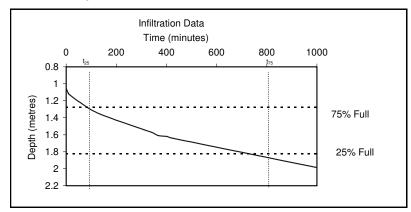
Site: Hallam Fields, Birstall, Leicester.

Client: Jelson Ltd

Page 2 of 2

Trialpit No.: SK 3

Soakaway Test Run 2 Test Date: 23/10/2003



Time	Depth
(minutes)	(m)
0	1.05
8	1.11
10	1.12
14	1.13
51	1.21
130	1.35
310	1.54
341	1.57
368	1.61
402	1.62
424	1.64
1358	2.20

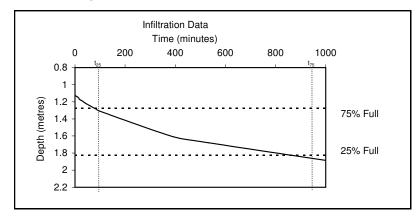
From the above graph,

$$t_{p25}$$
= 80 (min) t_{p75} = 795 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.45E-06

$$f_{run2} = 1.49 \times 10^{-6}$$
 m/s

Soakaway Test Run 3 Test Date: 24/10/2003



Time	Depth
(minutes)	(m)
0	1.12
2	1.13
8	1.14
13	1.15
23	1.18
28	1.19
43	1.22
80	1.28
394	1.61
533	1.68
1533	2.12
•	

From the above graph,

$$t_{p25}$$
= 85 (min) t_{p75} = 950 (min)

Soil Infiltration Rate:
$$f = V_{p75-25} \times N$$
 = 1.20E-06 $f_{run3} = 1.23 \times 10^{-6}$ m/s

Test and analysis carried out in general accordance with BRE Digest 365: 1991

Job No.: G04003

Site: Hallam Fields, Birstall, Leicester.

Client: Jelson Ltd

APPENDIX E

<u>LABORATORY TEST RESULTS - GEOTECHNICAL</u>

LR: G04003

SITE: Hallam Fields, Birstall.

NATURAL MOISTURE CONTENT AND ATTERBERG LIMIT TEST RESULTS

Tests carried out in accordance with B.S.1377: Part 2: 1990

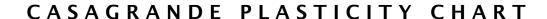
Hole No.	Depth (m)	Natural Moisture Content (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Passing 425 µm Sieve (%)	Group Symbol
BH1	1.20-1.65	15	32	16	16	100	CL
вн2	1.20-1.65	17	50	20	30	98	CI
BH4	2.00-2.40	15	39	17	22	92	CI
TP1	1.20-1.30	15	45	18	27	98	CI
TP12	1.10-1.20	13	34	16	18	98	CL
TP18	1.00-1.10	17	45	19	26	96	CI
TP39	1.40-1.50	20	42	18	24	98	CI

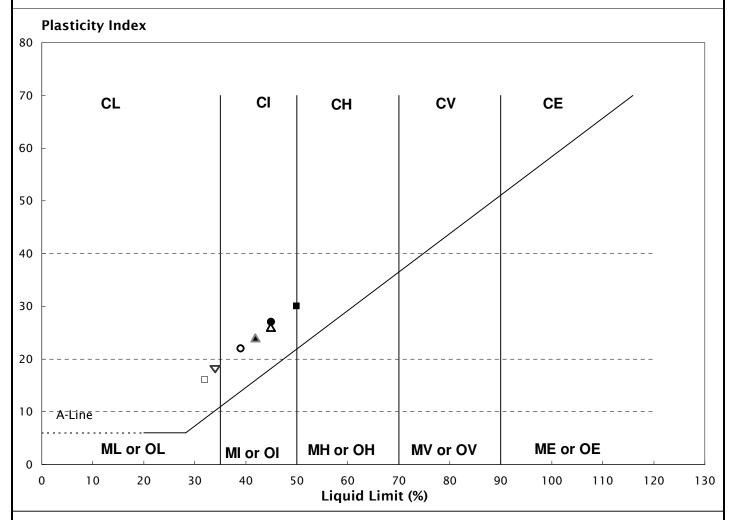
BH = Borehole; TP = Trial Pit

NICHOLLS COLTON GEOTECHNICAL

LR: G04003

DATE: November 2003





LEGENDS

★ # # □ BH1 ■ BH2 • BH4 • TP1 ▼ TP12 ▲ TP18 ▲ TP39

CASAGRANDE CLASSIFICATION

The fine grained soils are divided into the following groups and further subdivided on the basis of Liquid Limits:

SOIL GROUPS

C Inorganic clays (these plot *above* the A-Line)

M Inorganic silts (these plot *below* the A-Line)

O Organic silts and clays (these plot below the A-Line)

PLASTICITY SUB-GROUPS

L Low: less than 35%

I Intermediate: 35% to 50%

H High: 50% to 70%

V Very high: 70% to 90%

E Extremely high: more than 90%

SHRINKAGE POTENTIAL

NHBC 'Building Near Trees' Chapter 4.2

classifies shrinkable soils as having more than 35% fine particles (silt and clay) and further subdivides such soils into four categories of heave and shrinkage potential based on Plasticity Index:

HIGH PI > 40 **MEDIUM** PI = 20 - 40 **LOW** PI = 10 - 20 **NON-SHRINKABLE** PI < 10

Remarks:

Project: Hallam Fields, Birstall.

Project No.

G04003

Client: Jelson Ltd

SITE: Hallam Fields, Birstall.

SOIL SULPHATE CONTENT & pH VALUE RESULTS

Tests carried out in accordance with B.S.1377: Part 3: 1990

Hole No.	Sample Depth (m)	Description of sample	Sulphate Content (g/l as SO ₄)	pH Value	DS Class:	ACEC Class: *1
BH1	1.20-1.65	Slightly sandy slightly gravelly CLAY	<0.1	7.8	DS-1	AC-1s
вн2	1.20-1.65	Slightly gravelly CLAY	0.1	7.9	DS-1	AC-1s
вн3	1.20-1.50	Slightly gravelly CLAY	0.1	8.0	DS-1	AC-1s
BH4	2.00-2.40	Slightly gravelly CLAY	<0.1	8.2	DS-1	AC-1s
ВН5	2.50-2.95	Slightly silty slightly gravelly SAND	<0.1	8.7	DS-1	AC-1s
вн6	1.20-1.65	Slightly gravelly CLAY	<0.1	8.2	DS-1	AC-1s
вн7	3.00-3.45	Slightly sandy slightly gravelly CLAY	<0.1	8.3	DS-1	AC-1s
ВН8	2.50-3.00	Silty SAND	<0.1	8.4	DS-1	AC-1s
TP2	1.00-1.10	Slightly gravelly CLAY	<0.1	8.3	DS-1	AC-1s
TP3	1.00-1.10	Slightly gravelly CLAY	<0.1	8.3	DS-1	AC-1s
TP6	1.00-1.10	Slightly gravelly CLAY	<0.1	8.2	DS-1	AC-1s
TP7	0.50-0.60	Slightly gravelly CLAY	0.1	7.3	DS-1	AC-1s
TP11	1.10-1.20	Slightly clayey slightly silty SAND	<0.1	7.2	DS-1	AC-1s
TP12	1.10-1.20	Sandy CLAY	0.1	7.1	DS-1	AC-1s
TP15	1.00-1.10	Slightly gravelly CLAY	<0.1	7.8	DS-1	AC-1s
TP18	1.00-1.10	Slightly gravelly CLAY	<0.1	7.7	DS-1	AC-1s
TP20	1.00-1.10	Slightly gravelly CLAY	<0.1	8.0	DS-1	AC-1s
TP22	2.50-2.60	Gravelly SAND	<0.1	7.9	DS-1	AC-1s

Note: Results refer to water-soluble sulphates (2:1 Water:Soil Extract) unless otherwise specified.

*\frac{1}{2} - Classification determined from BRE Special Digest 1: 2001, Table 2

BH = Borehole; TP = Trial Pit

NICHOLLS COLTON GEOTECHNICAL LR: G04003

DATE: November 2003

SITE: Hallam Fields, Birstall.

SOIL SULPHATE CONTENT & pH VALUE RESULTS

Tests carried out in accordance with B.S.1377: Part 3: 1990

Hole No.	Sample Depth (m)	Description of sample	Sulphate Content (g/l as SO ₄)	pH Value	DS Class: *1	ACEC Class: *1
TP27	0.80-0.90	SILT	<0.1	8.1	DS-1	AC-1s
TP30	1.00-1.10	CLAY	<0.1	7.3	DS-1	AC-1s
TP36	1.20-1.30	Gravelly CLAY	<0.1	7.8	DS-1	AC-1s
TP39	1.40-1.50	Slightly gravelly CLAY	<0.1	8.1	DS-1	AC-1s
TP42	1.20-1.30	CLAY	<0.1	8.0	DS-1	AC-1s
TP47	1.20-1.30	Slightly gravelly SAND	<0.1	7.3	DS-1	AC-1s
TP49	1.20-1.30	CLAY	<0.1	6.9	DS-1	AC-1s

Note: Results refer to water-soluble sulphates (2:1 Water:Soil Extract) unless otherwise specified.

TP = Trial Pit

NICHOLLS COLTON GEOTECHNICAL

DATE: November 2003

LR: G04003

^{*1 -} Classification determined from BRE Special Digest 1: 2001, Table 2

SITE: Hallam Fields, Birstall.

SINGLE STAGE QUICK UNDRAINED TRIAXIAL COMPRESSION TEST RESULTS

Tests carried out in accordance with B.S.1377: Part 7: 1990

					Fai	lure Conditi	ons
Hole No.	Sample Depth	Natural Moisture	Bulk Density	Cell Pressure	Deviator Stress	Strain	Shear Strength
NO.	Берш	Content	Delisity	1 iessuie	Sucss		c_{u}
	(m)	(%)	(Mg/m^3)	(kN/m^2)	(kN/m^2)	(%)	(kN/m^2)
BH1	1.20-1.65	15	2.07	25	244	4.0	122
BH1	3.50-3.95	15	2.20	60	488	7.0	244
BH2	1.20-1.65	17	2.08	25	362	7.0	181
ВН2	2.00-2.45	19	2.13	40	303	21	152
вн3	1.20-1.50	17	2.15	40	485	7.5	243
вн3	3.00-3.35	17	2.23	60	520	4.5	260
BH4	2.00-2.40	15	2.18	40	498	5.0	249
BH4	4.00-4.45	15	2.23	80	493	6.0	247
ВН6	1.20-1.65	18	2.13	25	404	4.5	202
ВН6	2.00-2.45	20	2.17	40	287	18	143
ВН7	3.00-3.45	10	2.31	60	221	5.5	111
вн7	5.00-5.45	25	2.04	100	133	11	66

Nominal sample diameter = 105mm unless otherwise specified BH = Borehole

NICHOLLS COLTON GEOTECHNICAL

LR: G04003

DATE: November 2003

SITE: Hallam Fields, Birstall.

CALIFORNIA BEARING RATIO (C.B.R.) TEST RESULTS

Surcharge load: 16kg equivalent to approximately 450mm of road construction. Sample prepared at natural moisture content adopting Dynamic Compaction Method 5 (4.5 kg Rammer) Clause 7, B.S.1377: Part 4: 1990

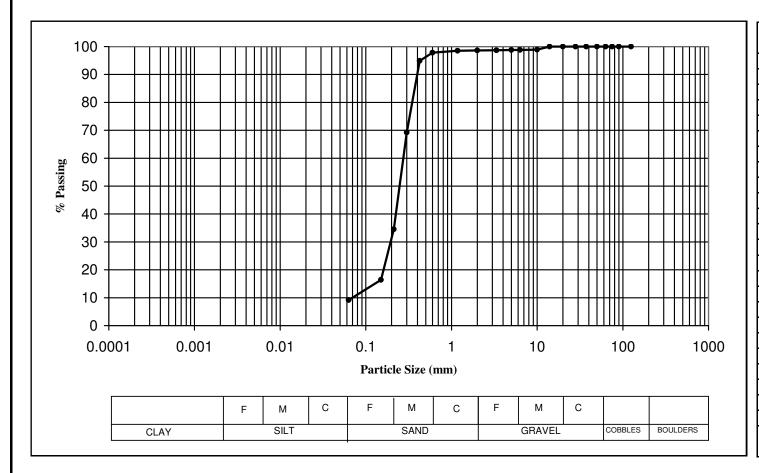
Hole	Sample	Sample	Moisture	Dry	CBR
No.	Depth	Description	Content	Density	
	(m)		(%)	(Mg/m ³)	(%)
TP3	0.50-0.60	Slightly gravelly CLAY	13	1.88	50
TP7	0.50-0.60	Slightly gravelly CLAY	17	1.80	18
TP9	0.50-0.60	Slightly gravelly CLAY	14	1.88	36
TP10	0.50-0.60	Silty fine SAND	11	2.01	34
TP18	0.50-0.60	Slightly gravelly CLAY	17	1.83	22
TP23	0.50-0.60	Slightly gravelly CLAY	16	1.84	30
TP28	0.50-0.60	Slightly silty slightly gravelly fine to medium SAND	10	2.01	16
TP31	0.50-0.60	Silty slightly gravelly fine to coarse SAND	4.5	1.87	78
TP37	0.50-0.60	Slightly gravelly CLAY	14	1.88	28
TP39	0.50-0.60	Slightly gravelly CLAY	19	1.76	16
TP47	0.50-0.60	Slightly clayey silty slightly gravelly fine SAND	5.9	1.98	>70
TP50	0.50-0.60	Silty slightly gravelly fine SAND	5.8	1.96	>107

TP = Trial Pit

NICHOLLS COLTON GEOTECHNICAL LR: G04003

DATE: November 2003

B.S.1377 1990 PART 2 PARTICLE SIZE ANALYSIS

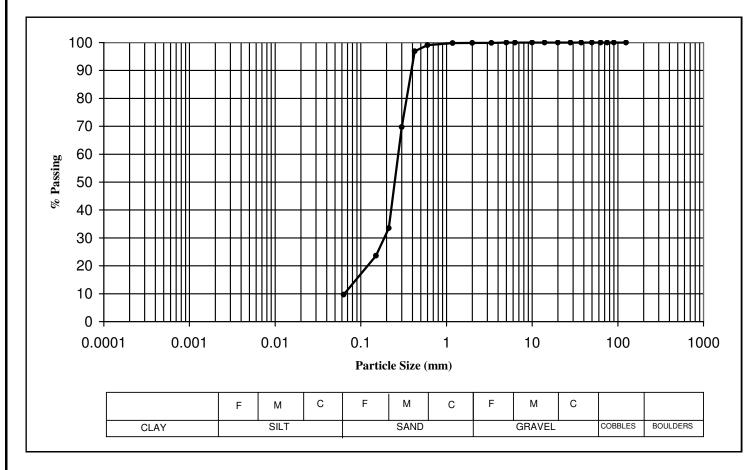


Sieve	Percentage	Group
Size	Passing	% retained
125	100	Cobbles
90	100	
75	100	
63	100	0
50	100	Gravel
37.5	100	
28	100	
20	100	
14	100	
10	99	
6.3	99	
5	99	
3.35	99	1
2	99	Sand
1.18	99	
0.6	98	
0.425	95	
0.3	69	
0.212	35	
0.15	16	
0.063	9	89
		Fines
		9

Key: F - Fine; M - Medium; C - Coarse

	Site Ref:	Job No:	G04003	Sample No:	B7	Date:
N.C.T	Hallam Fields, Birstall	Hole No.	BH 5	Depth (m):	2.50-2.95	October 2003

B.S.1377 1990 PART 2 PARTICLE SIZE ANALYSIS

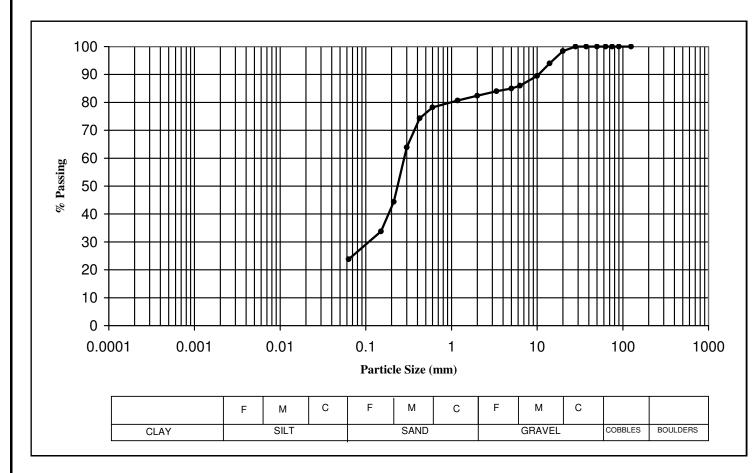


Sieve	Percentage	Group
Size	Passing	% retained
125	100	Cobbles
90	100	
75	100	
63	100	0
50	100	Gravel
37.5	100	
28	100	
20	100	
14	100	
10	100	
6.3	100	
5	100	
3.35	100	0
2	100	Sand
1.18	100	
0.6	99	
0.425	97	
0.3	70	
0.212	34	
0.15	24	
0.063	10	90
		Fines
		10

Key: F - Fine; M - Medium; C - Coarse

	Site Ref:	Job No:	G04003	Sample No:	B8	Date:
N.C.T	Hallam Fields, Birstall	Hole No.	BH 8	Depth (m):	2.50-3.00	October 2003

B.S.1377 1990 PART 2 PARTICLE SIZE ANALYSIS



Sieve	Percentage	Group
Size	Passing	% retained
125	100	Cobbles
90	100	
75	100	
63	100	0
50	100	Gravel
37.5	100	
28	100	
20	98	
14	94	
10	90	
6.3	86	
5	85	
3.35	84	18
2	82	Sand
1.18	81	
0.6	78	
0.425	74	
0.3	64	
0.212	44	
0.15	34	
0.063	24	59
		Fines
		24

Key: F - Fine; M - Medium; C - Coarse

N.C.T	Hallam Fields, Birstall	Hole No.	TP 47	Depth (m):	0.50-0.60	October 2003
	Site Ref:	Job No:	G04003	Sample No:	B3	Date:

APPENDIX F

LABORATORY TEST RESULTS - CONTAMINATION

LR: G04003





Soil Analysis

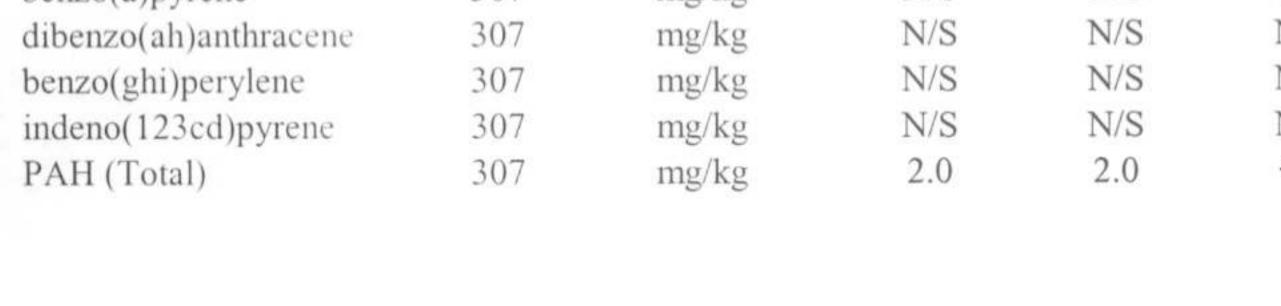
NCP /84965 Hallam Fields

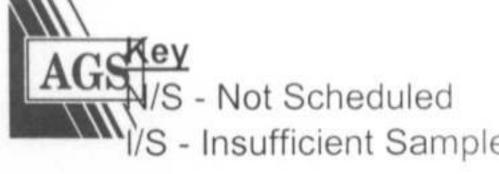
Your Reference:- G04003 Your Order:- NCP/7829

CAS Number:			427606	427607	427608	427609
Sample Ref			TP3	TP3	TP10	TP10
Detname	Method	Units	0.10m	$0.50 \mathrm{m}$	$0.10 \mathrm{m}$	0.40m
Arsenic (Total)	30/30C	mg/kg	14	18	10	8.9
Boron (Soluble)	6	mg/kg	0.76	0.47	0.13	0.27
Cadmium (Total)	30	mg/kg	0.55	< 0.50	0.62	< 0.50
Chromium (Hexavalent)	30B	mg/kg	< 0.10	< 0.10	0.18	N/S
Chromium (Total)	30	mg/kg	30	26	29	19
Copper (Total)	30	mg/kg	18	19	26	13
Lead (Total)	30	mg/kg	18	9.5	28	8.9
Mercury (Total)	30C	mg/kg	< 0.10	< 0.10	0.14	< 0.10
Nickel (Total)	30	mg/kg	28	32	19	17
Selenium (Total)	30C	mg/kg	0.28	0.34	0.24	0.16
Zinc (Total)	30	mg/kg	72	52	71	54
Cyanide (Total)	14	mg/kg	< 0.50	< 0.50	< 0.50	< 0.50
Organic Matter	36	0/0	N/S	N/S	N/S	N/S
Phenols (Total)	40A	mg/kg	< 0.50	< 0.50	< 0.50	< 0.50
Sulphate (Total) as SO4	45	%	0.020	< 0.01	0.020	< 0.01
Sulphide as S	47	mg/kg	< 5.0	< 5.0	< 5.0	< 5.0
рН	39	pH units	7.3	7.7	7.4	7.2
Sulphur (Elemental)	51	mg/kg	< 100	< 100	< 100	< 100
>> PAH SUITE <<			N/S	N/S	N/S	N/S
naphthalene	307	mg/kg	N/S	N/S	N/S	N/S
acenaphthylene	307	mg/kg	N/S	N/S	N/S	N/S
acenaphthene	307	mg/kg	N/S	N/S	N/S	N/S
fluorene	307	mg/kg	N/S	N/S	N/S	N/S
phenanthrene	307	mg/kg	N/S	N/S	N/S	N/S
anthracene	307	mg/kg	N/S	N/S	N/S	N/S
fluoranthene	307	mg/kg	N/S	N/S	N/S	N/S
pyrene	307	mg/kg	N/S	N/S	N/S	N/S
benzo(a)anthracene	307	mg/kg	N/S	N/S	N/S	N/S
chrysene	307	mg/kg	N/S	N/S	N/S	N/S
benzo(b)fluoranthene	307	mg/kg	N/S	N/S	N/S	N/S
benzo(k)fluoranthene	307	mg/kg	N/S	N/S	N/S	N/S
benzo(a)pyrene	307	mg/kg	N/S	N/S	N/S	N/S
dibenzo(ah)anthracene	307	mg/kg	N/S	N/S	N/S	N/S
benzo(ghi)perylene	307	mg/kg	N/S	N/S	N/S	N/S
indeno(123cd)pyrene	307	mg/kg	N/S	N/S	N/S	N/S
PAH (Total)	307	mg/kg	2.0	2.0	< 2	< 2











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Soil Analysis

PAGE 3 OF 4

NCP /84965 Hallam Fields

Your Reference:- G04003 Your Order:- NCP/7829

CAS Number:			427610	427611	427612	427613
Sample Ref			TP23	TP23	TP39	TP39
Detname	Method	Units	0.10m	$0.50 \mathrm{m}$	0.10m	$0.50 \mathrm{m}$
Arsenic (Total)	30/30C	mg/kg	15	18	10	14
Boron (Soluble)	6	mg/kg	0.58	0.45	0.46	0.44
Cadmium (Total)	30	mg/kg	0.59	< 0.50	< 0.50	< 0.50
Chromium (Hexavalent)	30B	mg/kg	0.13	< 0.10	0.12	< 0.10
Chromium (Total)	30	mg/kg	35	36	39	36
Copper (Total)	30	mg/kg	28	23	18	19
Lead (Total)	30	mg/kg	22	9.9	26	9.0
Mercury (Total)	30C	mg/kg	0.10	< 0.10	0.16	< 0.10
Nickel (Total)	30	mg/kg	29	44	18	32
Selenium (Total)	30C	mg/kg	0.34	0.42	0.25	0.43
Zinc (Total)	30	mg/kg	86	76	64	58
Cyanide (Total)	14	mg/kg	< 0.50	< 0.50	< 0.50	< 0.50
Organic Matter	36	%	4.2	1.2	N/S	N/S
Phenols (Total)	40A	mg/kg	< 0.50	< 0.50	< 0.50	< 0.50
Sulphate (Total) as SO4	45	%	0.030	< 0.01	0.10	< 0.01
Sulphide as S	47	mg/kg	< 5.0	< 5.0	< 5.0	< 5.0
рН	39	pH units	7.9	7.6	6.8	7.0
Sulphur (Elemental)	51	mg/kg	< 100	< 100	< 100	< 100
>> PAH SUITE <<					N/S	N/S
naphthalene	307	mg/kg	0.88	1.8	N/S	N/S
acenaphthylene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
acenaphthene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
fluorene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
phenanthrene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
anthracene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
fluoranthene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
pyrene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
benzo(a)anthracene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
chrysene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
benzo(b)fluoranthene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
benzo(k)fluoranthene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
benzo(a)pyrene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
dibenzo(ah)anthracene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
benzo(ghi)perylene	307	mg/kg	< 0.50	< 0.50	N/S	N/S
	307	mg/kg	< 0.50	< 0.50	N/S	N/S
indeno(123cd)pyrene	307	1115 115	0.0			





AGSKey N/S - Not Scheduled N/S - Insufficient Sample



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Soil Analysis

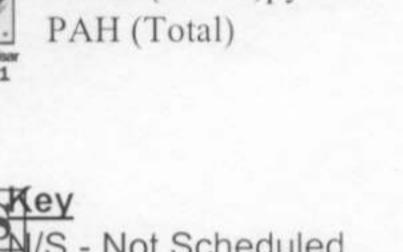
NCP /84965 Hallam Fields

Your Reference:- G04003 Your Order:- NCP/7829

CAS Number:			427614	427615	
Sample Ref			TP47	TP47	
Detname	Method	Units	0.10m	0.40m	
Arsenic (Total)	30/30C	mg/kg	9.2	10	
Boron (Soluble)	6	mg/kg	0.18	< 0.13	
Cadmium (Total)	30	mg/kg	< 0.50	< 0.50	
Chromium (Hexavalent)	30B	mg/kg	0.52	0.28	
Chromium (Total)	30	mg/kg	37	25	
Copper (Total)	30	mg/kg	24	16	
Lead (Total)	30	mg/kg	41	21	
Mercury (Total)	30C	mg/kg	0.23	0.14	
Nickel (Total)	30	mg/kg	16	15	
Selenium (Total)	30C	mg/kg	0.21	0.22	
Zinc (Total)	30	mg/kg	70	45	
Cyanide (Total)	14	mg/kg	< 0.50	< 0.50	
Organic Matter	36	%	N/S	N/S	
Phenols (Total)	40A	mg/kg	< 0.50	< 0.50	
Sulphate (Total) as SO4	45	%	0.010	< 0.01	
Sulphide as S	47	mg/kg	< 5.0	< 5.0	
рН	39	pH units	7.4	7.3	
Sulphur (Elemental)	51	mg/kg	< 100	< 100	
>> PAH SUITE <<			N/S	N/S	
naphthalene	307	mg/kg	N/S	N/S	
acenaphthylene	307	mg/kg	N/S	N/S	
acenaphthene	307	mg/kg	N/S	N/S	
fluorene	307	mg/kg	N/S	N/S	
phenanthrene	307	mg/kg	N/S	N/S	
anthracene	307	mg/kg	N/S	N/S	
fluoranthene	307	mg/kg	N/S	N/S	
pyrene	307	mg/kg	N/S	N/S	
benzo(a)anthracene	307	mg/kg	N/S	N/S	
chrysene	307	mg/kg	N/S	N/S	
benzo(b)fluoranthene	307	mg/kg	N/S	N/S	
benzo(k)fluoranthene	307	mg/kg	N/S	N/S	
benzo(a)pyrene	307	mg/kg	N/S	N/S	
dibenzo(ah)anthracene	307	mg/kg	N/S	N/S	
benzo(ghi)perylene	307	mg/kg	N/S	N/S	
indeno(123cd)pyrene	307	mg/kg	N/S	N/S	
PAH (Total)	307	mg/kg	< 2	< 2	









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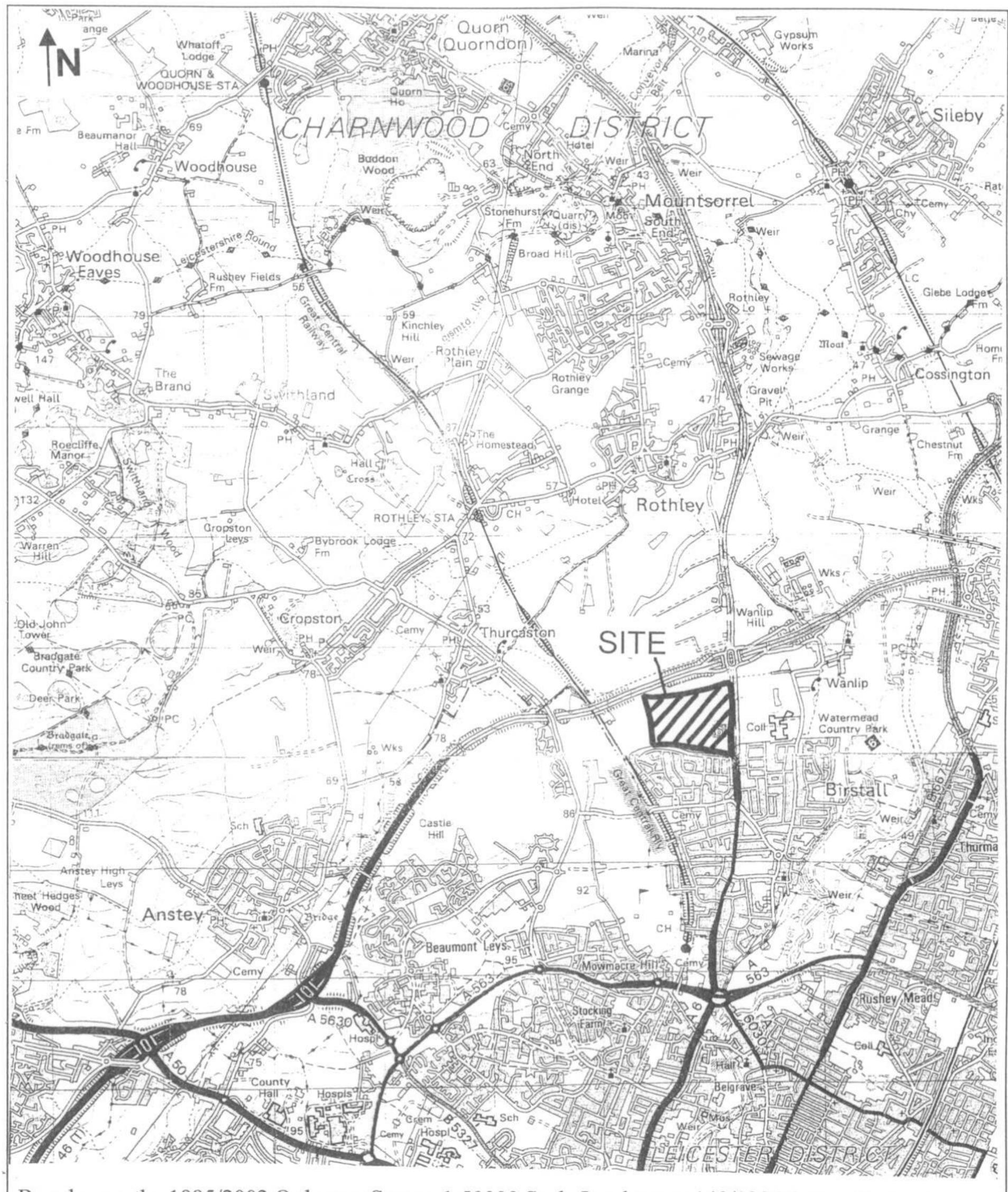
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DRAWINGS

G04003/01 - Site Location Plan

G04003/02 - Exploratory Hole Location Plan

LR: G04003



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NICHOLLS COLTON GEOTECHNICAL 7-11 Harding Street Leicester LE1 4DH

Client. Jelson Limited

Project.

Hallam Fields, Birstall, Leicester.

