

**Bridge Design Specification** 

Bridge No. BB1543

Date: January 2023

**Project Name:** Cross Keys Bridge

**Document Title:** Bridge Design Specification

**Document No:** BB1543-SP-001

Version	Issue	Prepared by: AK Date: 05/06/23	Checked by: ED Date: 05/06/23
C01	First Issue	f. John	Ed Nuh

#### 1. Introduction

This document is a performance specification for the proposed Cross Keys Footbridge to cross the River Rawthey in the Yorkshire Dales. The bridge will be a 35m span half-through pratt truss bridge and will replace an existing timber footbridge.

#### 2. General

#### 2.1 Site Details

Dimensions of the bridge is shown on drawing. BB1543-GA-001 Rev A. Grid reference for the bridge is SD 6982 9696 (54.3671476 -2.4644174) and the elevation of the bridge deck is approximately 199m AOD.

#### 2.2 Geometry

The bridge is to have the following critical dimensions:

2.2.1	Overall Length	35.750m
2.2.2	Span between bearings	35.000m
2.2.3	Clear Width Between Parapets	2.500m
2.2.4	Parapet Height Above Deck	1.800m
2.2.5	Overall Width	2.950m
2.2.6	Construction Depth	300mm

### 2.3 Design Life

Design working lives for bridge components will be as follows as per CD 350:

Superstructure: Steel bridge will have a design life of 120 years (Category 5) Bearings: Elastomeric bearings will have a design life of 15 years (Category 2) Parapets: Softwood timber will have a design life of 15 years (Category 2) Decking: FRP panels will have a design life of 50 years (Category 3)

#### 2.4 Classes and Levels

Consequence class to be CC2 in accordance with BS EN 1990:2002+A1:2005. Reliability class to be RC2 in accordance with BS EN 1990:2002+A1:2005. Inspection level to be IL2 in accordance with BS EN 1990:2002+A1:2005.

### 2.5 Materials

#### <u>Superstructure Steelwork:</u>

All steelwork will be minimum steel grade S355 to BS EN 10025 or an equivalent higher grade. All steelwork greater than 4mm

thick will have a minimum sub-grade of JO.

### **Corrosion Protection:**

All steelwork to be applied with a Highways Agency Type II Paint System in accordance with The Specification for Highway Works Series 1900 or a fluoropolymer (FEVE) system such as Vitreflon 195. Top coat colour RAL 7022 Umba Grey.

#### FRP:

The FRP deck panels are durable and maintenance free with anti-skid surfacing with a slip resistance in accordance with CD 353 clause 9.5. All FRP pultruded products to be minimum E23 grade.

### Concrete:

Element	Strength Class in accordance with BS8500-1	Design Chemical Class	Exposure Class
Abutments	C28/35	DC1	XD3

### Concrete finishes:

Element	Finish
Buried unformed surfaces	U1
Buried/hidden formed surfaces	F1
Exposed formed surfaces	F3
Exposed unformed surfaces	U3

#### Structural backfill:

Class 6N/6P to abutments.

#### 2.6 Articulation

All bearings will be Type C elastomeric deforming bearings C200200(10.0)23C5 supplied by SKE Bearings. The bearings are designed to permit longitudinal movement and deformations only whereby transverse movement and deformations will be resisted by vertical guided plates .

#### 2.7 Road and Pedestrian Restraint Requirements

The pedestrian restraint system is 1.8m high, formed from horizontal timber rails spanning between the vertical and diagonal brace members of the truss. The top of truss top chord will act as top rail at 1.8m from deck level.

The rails will provide a 600mm infill panel at 25mm above deck level in line with the requirements for equestrian use according to BS 7818, CD353 and CD377. Guidance from Path Bridges (Forestry Commission) recommends the horizontal rails to be designed for a minimum load of 1.3kN/m and 0.74kN/m for equestrian use and pedestrian normal loading, respectively. This also corresponds to BS 7817 which recommends Class 2 loading (0.7kN/m) for normal duty guard rails.

#### 2.8 Foundations

The foundations to both embankments will be designed as shallow bearing reinforced concrete abutments.

The supports for the bridge will be designed and constructed to a tolerance of + or - 20mm in line in any direction and + or - 10mm in level. The bridge shall be designed and fabricated such that it can still function and be installed even if the supports are constructed up to these tolerances away from their intended location.

Differential settlement between any 2 foundations could be up to 20mm. The bridges should still be able to function with this level of differential settlement between supports.

#### 3. Design Criteria

#### 3.1 Permanent Actions

#### 3.1.1 Self-weight of used materials (CS454 Table 4.1.1a)

Steel 7850 kg/m<sup>3</sup>

Concrete 2400 kg/m<sup>3</sup>

### 3.2 Variable Actions

#### 3.2.1 Wind actions on bridge

Defined according to BS EN 1991-1-4 and applied in accordance with Section 8. Basic hourly wind speed from NA to BS EN 1991-1-4 Figure NA.1 = 23.5 m/s.

#### 3.2.2 Snow Actions

Snow actions will not be considered as per NA to BS EN 1990 NA.2.3.4.2 A2.2.3(3).

#### 3.2.3 Temperature Actions

Defined according to BS EN 1991-1-5 and applied in accordance with Section 6.

Minimum shade air temperature (Figure NA.1) = -17°C

Maximum shade air temperature (Figure NA.2) = +31°C

#### 3.2.4 Seismic Actions

The effects of seismicity have not been considered due to the low consequence of the structure.

#### 3.2.5 Hydrodynamic Actions

Due to the soffit level of the bridge, there is no requirement for hydrodynamic actions to be considered.

#### 3.2.6 Accidental Actions

Not applicable.

#### 3.2.7 Construction Actions

The bridge will be designed for actions during its construction (i.e. lifting in).

#### 3.2.8 Carriageway Actions

#### 3.2.8.1 Uniformly Distributed Loads

Pedestrian loading on the structure will be considered by applying uniformly distributed load in accordance with NA.2.36 of NA to BS EN 1991-2.

#### 3.2.8.2 Vehicle Traffic Load

Not applicable.

#### 3.2.9 Dynamic Loads

#### 3.2.9.1 Pedestrian Comfort Criteria

Detailed dynamic analysis will be performed for fundamental frequencies below values specified BSEN1990 Annex 2 A.2.4.3.2 ii): 5 Hz for vertical direction and 2.5 Hz for horizontal direction.

#### 3.2.9.2 Crowd Density

For bridge class for design crowd density determination, Class A will be assumed as per NA to BS EN 1991-2, Table NA.7.

#### 3.2.9.3 Limits for Dynamic Response

Value for dynamic response limit of 1.23 m/s $^2$  will be used (based on NA.2.44.6 and Figure NA.10 of NA to BS EN 1991-2). This factor has been developed assuming: k1 = 1.6 (Rural)

k2 = 0.7 (Primary Route. No other routes are available)

k3 = 1.1 (Height above flood channel is less than 4m)

k4 = 1.0 (Recommended value)

Appendix A – Geotechnical Investigations



## **GEO2020-4541: Ground Investigation Report (Factual)**

## Footbridge, Cross Keys, Sedbergh

#### **Introduction:**

The site comprises a footbridge at Cross Keys, Sedbergh.

Cross Keys Footbridge, A683, Sedbergh, Cumbria
 The Cross Keys Temperance Inn, A683, Sedbergh, South Lakeland, Cumbria, LA10 5NE
 NGR: 369820, 496960 (SD6982 9696)

The proposal is to replace the footbridge. GEO was requested to undertake the investigation by RG Parkins Consulting Engineers on behalf of Yorkshire Dales National Park Authority.

#### **Previous Investigation Works:**

GEO is not aware of any previous investigation works.

#### **Fieldworks:**

GEO were requested to undertake two rotary cored boreholes to determine ground and groundwater conditions to aid the design of the replacement bridge by the Structural Engineer. Any items not specifically mentioned cannot be assumed to be covered.

The site had restricted access, with the northern bank accessible by foot only. RBH01 was completed by a rotary coring rig, with the borehole positioned within the farmyard adjacent to the river and bridge. RBH02 was completed using hand-held coring equipment, with the borehole positioned on the north side of the river adjacent to the bridge.

#### Fieldworks Findings (GEO):

RBH01 noted made ground of topsoil with anthropogenic debris to c.0.30m bgl overlying dense, very gravelly sand to c.1.20m bgl. Dense becoming very dense very sandy, slightly clayey gravel with cobbles was noted to a depth of c.2.80m bgl. Dark grey SILTSTONE initially weathered and recovered as gravel was noted to c.4.30m bgl. Strong appearing dark grey SILTSTONE was encountered from c.4.30m to the base of the borehole at c.7.50m which was noted as more competent with depth. Core was recovered (100%) from c.4.50m to c.7.50m depth.

RBH02 noted rough grass over topsoil to c.0.35m depth. Siltstone gravel and cobbles was then present to c.0.65m bgl. Dark grey SILTSTONE, initially weathered and recovered as gravel was noted to c.1.20m bgl. From c.1.20m to the base of the borehole at c.1.50m dark grey SILTSTONE was noted. 1.50m bgl which was the operational limit of the handheld coring equipment.

#### **In-situ Geotechnical Testing:**

In-situ Standard Penetration tests were completed within RBH01, generally at c.1.00m spacings. The test values recorded on the borehole logs. Due to the handheld drilling techniques employed on the northern bank, in-situ testing was not possible.

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### In-situ Sampling:

Samples were recovered from the borehole and details are presented on the exploratory hole log. This included samples of the drift deposits, weathered rock and cores of the intact rock.

### **Laboratory Testing:**

Geotechnical Testing:

The following laboratory based geotechnical testing was undertaken by Pro Soils Laboratory Ltd of Doncaster:

- Determination of Unconfined Compressive Strength (4 no. samples)
- Determination of Point Load Tests (Axial and Diametral 3 no. samples)

The results of the testing are summarised below and presented in the Laboratory test reports within the appendices.

#### DETERMINATION OF UNCONFINED COMPRESSIVE STRENGTH

ISRM Suggested Methods, pp 111-116, 1981.

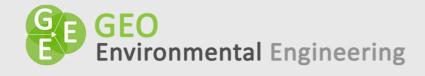
Hole		Sample		Base		Sample		Initial		Moisture		Load	UCS	Failure	Date	Remarks
Number	Number	Type	Depth		Diameter		Ratio	Mass	Density	Content	Density	Failure	(A.F.)	Mode	Tested	
			(m)	(m)	(mm)	(mm)		(g)	(Mg/m)	(%)	(Mg/m)	(kN)	(MPa)			
RBH01		C	4.60	4.75	47	93	2.0	439	2.72	1.4	2.68	21.0	12.1	Brittle	12/04/21	
RBH01		C	5.80	6.07	48	94	2.0	442	2.60	0.7	2.58	58.2	32.2	Brittle	12/04/21	
RBH01		C	6.50	7.10	48	97	2.0	461	2.63	1.0	2.60	50.1	27.7	Brittle	12/04/21	
RBH01		C	7.10	7.35	47	95	2.0	453	2.75	0.8	2.73	58.3	33.6	Brittle	12/04/21	



Cross Keys Footbridge, Sedbergh

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#### SUMMARY OF POINT LOAD TEST RESULTS

ISRM Suggested Methods: 2007

Borehole Number	Depth (m)	Sample Ref	Test Type	Orientation	Dimer (m	nsions m)	Area	D <sub>e</sub> <sup>2</sup>	D <sub>e</sub>	Failure	Load (P)	I,	Corr Fac	$I_{s50}$	Failure Type	Remarks
			-71	Par / Perp	w	D	(mm2)		(mm)	(Mpa)	(kN)	(MPa)	F	(MPa)	-71	
RBH01	5.00		I	Par	38	38	1444	1838.56	42.88	•	6.52	3.55	0.933	3.31	Valid	
RBH01	6.30		A	Perp	47	32	1504	1914.95	43.76	-	8.69	4.54	0.942	4.27	Valid	
														, and the second		

\*Note All testing carried out on samples at as received water content

 $Par = parallel, \, Perp = perpendicular, \, U = Random$ 

 $A = Axial, \, D = Diametral, \, I = Irregular$ 



Cross Keys Footbridge, Sedbergh

Co	ntract No:
PS	L21/2196
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20	020-4541

#### SUMMARY OF POINT LOAD TEST RESULTS

ISRM Suggested Methods: 2007

Borehole Number	Depth (m)	Sample Ref	Test Type	Orientation	Dimer (m		D <sub>e</sub> <sup>2</sup>	D <sub>e</sub>	Failur	e Load	I,	Corr Fac	$I_{s50}$	Failure Type	Remarks		
Number	(111)	Kei	Турс	Par / Perp	L	D		(mm)	(Mpa)	(kN)	(MPa)	F	(MPa)	Турс			
RBH01	6.30		D	Par	•	47	2209	47.00		9.76	4.418	0.973	4.30	Valid			
*Note	All testing of	carried out or	n samples a	at as received wa	ater conte	*Note All testing carried out on samples at as received water content Par = parallel, Perp = perpendicular, U = Random											



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Geochemical and Contamination Testing:

The following laboratory based geochemical and contamination testing was undertaken by Chemtech Environmental Limited of Stanley:

- Determination of Generic metalloid and inorganic contaminants (1 no. sample)
- Determination of pH and Water Soluble Sulphate (3 no. samples)

The results of the testing are summarised below and presented in the Laboratory test reports within the appendices.

#### **Chemtech Environmental Limited**

#### SOILS

Lab number			94487-1	94487-2	94487-3
Sample id			RBH01	RBH01	RBH01
Depth (m)			0.00-0.30	0.50-1.00	1.50-1.90
Date sampled			09/03/2021	09/03/2021	09/03/2021
Test	Method	Units			
Arsenic (total)	CE127 <sup>M</sup>	mg/kg As	20	-	
Cadmium (total)	CE127 <sup>M</sup>	mg/kg Cd	0.3	-	-
Chromium (total)	CE127 <sup>™</sup>	mg/kg Cr	72	-	-
Chromium (III)	CE208	mg/kg CrIII	72	-	-
Chromium (VI)	CE146	mg/kg CrVI	<1	-	-
Copper (total)	CE127 <sup>M</sup>	mg/kg Cu	23	-	-
Lead (total)	CE127 <sup>™</sup>	mg/kg Pb	116	-	-
Mercury (total)	CE127 <sup>™</sup>	mg/kg Hg	<0.5	-	-
Nickel (total)	CE127 <sup>™</sup>	mg/kg Ni	24	-	-
Selenium (total)	CE127 <sup>™</sup>	mg/kg Se	1.5	-	-
Zinc (total)	CE127 <sup>™</sup>	mg/kg Zn	128	-	-
рН	CE004 <sup>M</sup>	units	7.9	8.0	8.2
Sulphate (2:1 water soluble)	CE061	mg/I SO <sub>4</sub>	19	15	<10
Cyanide (total)	CE077	mg/kg CN	<1	-	-
Total Organic Carbon (TOC)	CE197	% w/w C	3.5	-	-

Lab number		94487-4	
Sample id	RBH01		
Date sampled	09/03/2021		
Test	Method	Units	
рН	CE160	units	8.4
Sulphate (2:1 water soluble)	CE061	mg/I SO <sub>4</sub>	53

#### **General Comments and Limitations:**

Consideration must be made for variations to occur in the ground conditions between the exploratory hole locations for which GEO holds no responsibility. It is therefore recommended that a "watching brief" and "observational technique" be applied to this site to ensure that if ground conditions appear to vary from those identified within this investigation report then advice should be sought from a suitably qualified and experienced Engineering Geologist, Geotechnical or Geo-Environmental Engineer.

The recommendations and opinions expressed in this report are based on the strata observed within the exploratory holes in addition to the results of the site and laboratory tests commissioned by GEO. Consequently, GEO takes no responsibility for conditions that have not been revealed or which occur between them. GEO takes no responsibility for the accuracy of third-party information provided by subcontract drillers.

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The conclusions and recommendations presented within this report are considered reasonable based on the available information. However, these cannot be guaranteed to gain regulatory approval. Therefore, the report should be passed to the appropriate regulatory authorities and/or other key stakeholders in order to seek their approval of the findings prior to undertaking any works on site.

### Factual Report Completed By:

Curtis R. Evans B.Sc (Hons) FGS

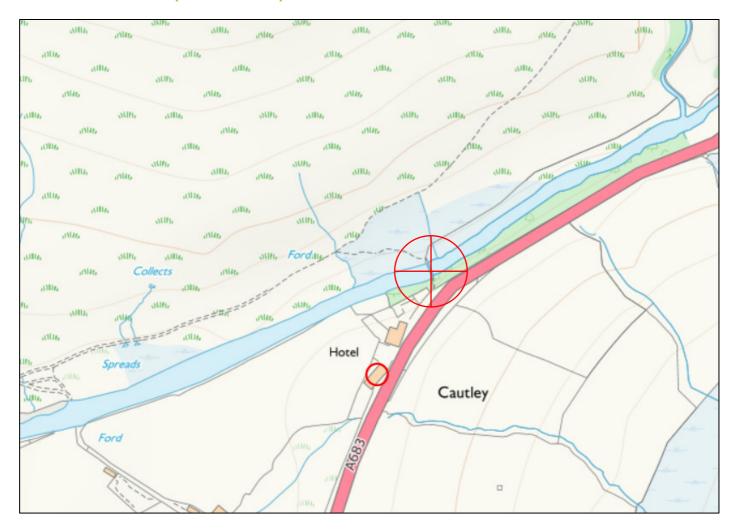
Director

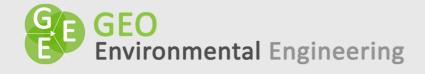
For and On Behalf Of Geo Environmental Engineering Ltd

Visit our Website: www.geoenvironmentalengineering.com Email: info@geoenvironmentalengineering.com

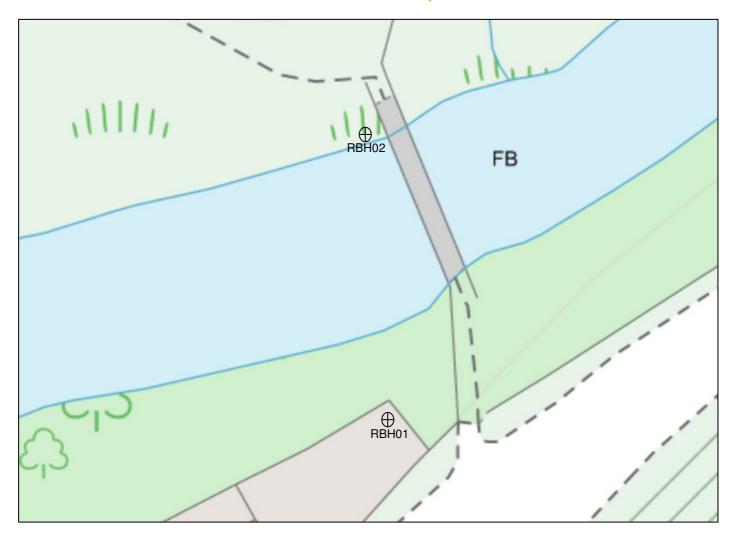


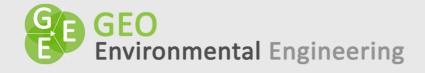
## **Site Location Plan (Not to Scale)**





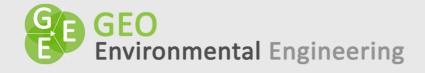
Exploratory Hole Location Plan (Approximate Locations – Not to Scale – Borehole Locations Positioned Where Safe Access Allowed)



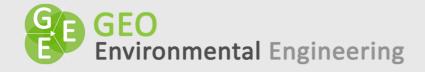


Depth	Depth	Strata		Legend	Testing /		
From (m)	To (m)	Description			Samples		
0.00	0.30	MADE GROUND: Dark brown very sandy fine to coarse gravel including brick and ma			0-0.30 TJ		
0.30	1.20	Dense dark brown very gravelly fine to coafine to coarse and angular of sandstor siltstone and occasional cobbles.			0.50-1.00 B 0.63 SPT: N37		
1.20	2.80	Dense becoming very dense grey brown, inivery sandy fine to coarse GRAVEL of ang limestone, sandstone, siltstone and slacobbles.		1.20 SPT: N31 1.50-1.90 B 1.90 SPT: N50/145mm			
2.80	4.30	Dark grey SILTSTONE. Weathered. Recover angular and tabular gravel.		3.00-4.00 B			
4.30	7.50	Strong appearing dark grey SILTSTONE. Occ quartz veining. Occasionally fractured Becoming more competent with depth.			4.50-5.50: 100% Recovery C		
					5.50-6.50: 100% Recovery C 6.50-7.50: 100% Recovery C		
Log Notes: End of boreh Borehole rer Full flush thr	mained ope	en and dry on completion		Hand dug Cased to 4	before drilling .50m.		
Site: Footbri Client: RG Pa Engineer: GE Site Works D	dge, Cross arkins E/CRE Date: Feb 2	Keys, Sedbergh	Log Key:  SPT = Standard Penetration Test (result as N value)  HSV = Hand Shear Vane (result in kN/m²)  CBR = California Bearing Ratio by Mexe Cone Penetrometer (resul as percentage)  LP = Limited Penetration (HSV/CBR)  NP = No penetration (HSV/CBR)  B = Bulk Bag, J = Amber Glass Jar, T = Plastic Tub  C = Core Run including %age recovery				

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Depth	Depth	Strata		Legend	Testing /		
From (m)	To (m)	Description			Samples		
0.00	0.35	Rough grass over dark brown sandy, gravel	ly TOPSOIL.				
0.35	0.65	Grey and brown sandy siltstone GRAVEL wit occasional boulders.	h many cobbles and				
0.65	1.20	Dark grey SILTSTONE. Weathered. Recover angular and tabular gravel.					
1.20	1.50	Hard dark grey SILTSTONE.					
Log Notes:				Hand dug	before core drilling to c.0.65m		
End of boreh	nole						
Borehole rer	mained ope	en and dry on completion					
Full flush thr							
	-	Keys, Sedbergh	Log Key:				
Client: RG Pa			SPT = Standard Pene		,		
Engineer: Gl	•		HSV = Hand Shear Vane (result in kN/m²)				
Site Works I			CBR = California Bearing Ratio by Mexe Cone Penetrometer (result				
Plant: Hycor	n coring rig	with water flush	as percentage)				
			LP = Limited Penetration (HSV/CBR)				
			NP = No penetration (HSV/CBR)				
			B = Bulk Bag, J = Amber Glass Jar, T = Plastic Tub				
			C = Core Run including %age recovery				



# **Core Sample Photographs**





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## **Core Sample Photographs**

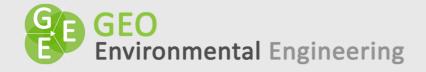




## **Site Works Photographs**



**Website:** www.geoenvironmentalengineering.com **Email:** info@geoenvironmentalengineering.com



## **Site Works Photographs**



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# LABORATORY REPORT



4043

Contract Number: PSL21/2196

Report Date: 13 April 2021

Client's Reference: 2020-4541

Client Name: GEO Environmental Engineering

4 Culgarth Avenue Cockermouth Cumbria CA13 9PL

For the attention of: Curtis Evans

Contract Title: Cross Keys Footbridge, Sedbergh

Date Received: 15/3/2021 Date Commenced: 15/3/2021 Date Completed: 13/4/2021

Notes: Opinions and Interpretations are outside the UKAS Accreditation

A copy of the Laboratory Schedule of accredited tests as issued by UKAS is attached to this report. This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced other than in full, without the prior written approval of the laboratory.

### Checked and Approved Signatories:

A Watkins R Berriman S Royle

(Director) (Quality Manager) (Laboratory Manager)

L Knight S Eyre H Daniels (Senior Technician) (Senior Technician) (Senior Technician)

Page 1 of

5 – 7 Hexthorpe Road, Hexthorpe,

Doncaster DN4 0AR tel: +44 (0)844 815 6641 fax: +44 (0)844 815 6642

e-mail: rberriman@prosoils.co.uk awatkins@prosoils.co.uk

# DETERMINATION OF UNCONFINED COMPRESSIVE STRENGTH

ISRM Suggested Methods, pp 111 –116, 1981.

Hole Number	Sample Number	Sample Type	Top Depth (m)	Base Depth (m)	Sample Diameter (mm)	Sample Length (mm)	Height Ratio	Initial Mass (g)	Bulk Density (Mg/m)	Moisture Content (%)	-	Load Failure (kN)	UCS (MPa)	Failure Mode	Date Tested	Remarks
RBH01		C	4.60	4.75	47	93	2.0	439	2.72	1.4	2.68	21.0	12.1	Brittle	12/04/21	
RBH01		C	5.80	6.07	48	94	2.0	442	2.60	0.7	2.58	58.2	32.2	Brittle	12/04/21	
RBH01		C	6.50	7.10	48	97	2.0	461	2.63	1.0	2.60	50.1	27.7	Brittle	12/04/21	
RBH01		C	7.10	7.35	47	95	2.0	453	2.75	0.8	2.73	58.3	33.6	Brittle	12/04/21	
RDIIOI			7.10	7.00		70	2.0		2010	0.0	20,70	2010	00.0	Dilette	12/01/21	
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PSL
Professional Soils Laboratory

Cross Keys Footbridge, Sedbergh

Contract No:
PSL21/2196
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# **SUMMARY OF POINT LOAD TEST RESULTS**

**ISRM Suggested Methods: 2007** 

Borehole Number	Depth (m)	Sample Ref	Test Type	Orientation	Dimei (m	m)	Area	D <sub>e</sub> <sup>2</sup>	D <sub>e</sub>		Load (P)	Is	Corr Fac	$I_{s50}$	Failure Type	Remarks
			• •	Par / Perp	W	D	(mm2)		(mm)	(Mpa)	(kN)	(MPa)	F	(MPa)		
RBH01	5.00		I	Par	38	38	1444	1838.56	42.88	-	6.52	3.55	0.933	3.31	Valid	
RBH01	6.30		A	Perp	47	32	1504	1914.95	43.76	-	8.69	4.54	0.942	4.27	Valid	
							-			-						

\*Note All testing carried out on samples at as received water content

Par = parallel, Perp = perpendicular, U = Random

A = Axial, D = Diametral, I = Irregular





Cross Keys Footbridge, Sedbergh

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# **SUMMARY OF POINT LOAD TEST RESULTS**

**ISRM Suggested Methods: 2007** 

Borehole Number	Depth (m)	Sample Ref	Test Type	Orientation	Dimei (m	nsions m)	D <sub>e</sub> <sup>2</sup>	D <sub>e</sub>	Failur	e Load	I <sub>s</sub>	Corr Fac	$I_{s50}$	Failure Type	Remarks
Tulliber	()	101	1,100	Par / Perp	L	D		(mm)	(Mpa)	(kN)	(MPa)	F	(MPa)	1,700	
RBH01	6.30		D	Par	-	47	2209	47.00	-	9.76	4.418	0.973	4.30	Valid	

\*Note All testing carried out on samples at as received water content

Par = parallel, Perp = perpendicular, U = Random



Cross Keys Footbridge, Sedbergh

Contract No:
PSL21/2196
Client Ref:
2020-4541







### **ANALYTICAL TEST REPORT**

Contract no: 94487

**Contract name:** Cross Keys Footbridge, Sedbergh

Client reference: GEO2020-4541

Clients name: Geo Environmental Engineering

Clients address: 4 Culgarth Avenue

Cockermouth Cumbria CA13 9PL

Samples received: 18 March 2021

Analysis started: 18 March 2021

Analysis completed: 25 March 2021

Report issued: 25 March 2021

**Notes:** Opinions and interpretations expressed herein are outside the UKAS accreditation scope.

Unless otherwise stated, Chemtech Environmental Ltd was not responsible for sampling.

All testing carried out at Unit 6 Parkhead, Stanley, DH9 7YB, except for subcontracted testing.

Methods, procedures and performance data are available on request.

Results reported herein relate only to the material supplied to the laboratory. This report shall not be reproduced except in full, without prior written approval. Samples will be disposed of 6 weeks from initial receipt unless otherwise instructed.

**Key:** U UKAS accredited test

M MCERTS & UKAS accredited test

\$ Test carried out by an approved subcontractor

I/S Insufficient sample to carry out test N/S Sample not suitable for testing

Approved by:

Rachael Burton

Customer Support Squad Leader

## **SAMPLE INFORMATION**

#### MCERTS (Soils):

Soil descriptions are only intended to provide a log of sample matrices with respect to MCERTS validation. They are not intended as full geological descriptions. MCERTS accreditation applies for sand, clay and loam/topsoil, or combinations of these whether these are derived from naturally occurring soils or from made ground, as long as these materials constitute the major part of the sample. Other materials such as concrete, gravel and brick are not accredited if they comprise the major part of the sample.

All results are reported on a dry basis. Samples dried at no more than  $30^{\circ}\text{C}$  in a drying cabinet. Analytical results are inclusive of stones.

Lab ref	Sample id	Depth (m)	Sample description	Material removed	% Removed	% Moisture
94487-1	RBH01	0.00-0.30	Loamy Clay with Gravel & Roots	-	-	25.3
94487-2	RBH01	0.50-1.00	Loamy Clay with Gravel & Roots	-	-	13.3
94487-3	RBH01	1.50-1.90	Loamy Clay with Gravel & Roots	-	-	7.2

# **SOILS**

Lab number			94487-1	94487-2	94487-3
Sample id			RBH01	RBH01	RBH01
Depth (m)			0.00-0.30	0.50-1.00	1.50-1.90
Date sampled			09/03/2021	09/03/2021	09/03/2021
Test	Method	Units			
Arsenic (total)	CE127 <sup>M</sup>	mg/kg As	20	-	-
Cadmium (total)	CE127 <sup>M</sup>	mg/kg Cd	0.3	-	-
Chromium (total)	CE127 <sup>M</sup>	mg/kg Cr	72	-	-
Chromium (III)	CE208	mg/kg CrIII	72	-	-
Chromium (VI)	CE146	mg/kg CrVI	<1	-	-
Copper (total)	CE127 <sup>M</sup>	mg/kg Cu	23	-	-
Lead (total)	CE127 <sup>M</sup>	mg/kg Pb	116	-	-
Mercury (total)	CE127 <sup>M</sup>	mg/kg Hg	<0.5	-	-
Nickel (total)	CE127 <sup>M</sup>	mg/kg Ni	24	-	-
Selenium (total)	CE127 <sup>M</sup>	mg/kg Se	1.5	-	-
Zinc (total)	CE127 <sup>M</sup>	mg/kg Zn	128	-	-
рН	CE004 <sup>M</sup>	units	7.9	8.0	8.2
Sulphate (2:1 water soluble)	CE061	mg/l SO <sub>4</sub>	19	15	<10
Cyanide (total)	CE077	mg/kg CN	<1	-	-
Total Organic Carbon (TOC)	CE197	% w/w C	3.5	-	-

# **SOLIDS**

Lab number		94487-4	
Sample id	RBH01		
Date sampled			09/03/2021
Test	Method	Units	
рН	CE160	units	8.4
Sulphate (2:1 water soluble)	CE061	mg/l SO₄	53

# **METHOD DETAILS**

METHOD	SOILS	METHOD SUMMARY	SAMPLE	STATUS	LOD	UNITS
CE127	Arsenic (total)	Aqua regia digest, ICP-MS	Dry	М	1	mg/kg As
CE127	Cadmium (total)	Aqua regia digest, ICP-MS	Dry	М	0.2	mg/kg Cd
CE127	Chromium (total)	Aqua regia digest, ICP-MS	Dry	М	1	mg/kg Cr
CE208	Chromium (III)	Calculation: Cr (total) - Cr (VI)	Dry		1	mg/kg CrIII
CE146	Chromium (VI)	Acid extraction, Colorimetry	Dry		1	mg/kg CrVI
CE127	Copper (total)	Aqua regia digest, ICP-MS	Dry	М	1	mg/kg Cu
CE127	Lead (total)	Aqua regia digest, ICP-MS	Dry	М	1	mg/kg Pb
CE127	Mercury (total)	Aqua regia digest, ICP-MS	Dry	М	0.5	mg/kg Hg
CE127	Nickel (total)	Aqua regia digest, ICP-MS	Dry	М	1	mg/kg Ni
CE127	Selenium (total)	Aqua regia digest, ICP-MS	Dry	М	0.3	mg/kg Se
CE127	Zinc (total)	Aqua regia digest, ICP-MS	Dry	М	5	mg/kg Zn
CE004	рН	Based on BS 1377, pH Meter	As received	М	-	units
CE061	Sulphate (2:1 water soluble)	Aqueous extraction, ICP-OES	Dry		10	mg/l SO <sub>4</sub>
CE077	Cyanide (total)	Extraction, Continuous Flow Colorimetry	As received		1	mg/kg CN
CE197	Total Organic Carbon (TOC)	Carbon Analyser	Dry		0.1	% w/w C

# **METHOD DETAILS**

METHOD	SOLIDS	METHOD SUMMARY	SAMPLE	STATUS	LOD	UNITS
CE004	рН	Based on BS 1377, pH Meter	As received		-	units
CE061	Sulphate (2:1 water soluble)	Aqueous extraction, ICP-OES	Dry		10	mg/l SO <sub>4</sub>

## **DEVIATING SAMPLE INFORMATION**

#### Comments

Sample deviation is determined in accordance with the UKAS note "Guidance on Deviating Samples" and based on reference standards and laboratory trials.

For samples identified as deviating, test result(s) may be compromised and may not be representative of the sample at the time of sampling.

Chemtech Environmental Ltd cannot be held responsible for the integrity of sample(s) received if Chemtech Environmental Ltd did not undertake the sampling. Such samples may be deviating.

Key

N No (not deviating sample)
Y Yes (deviating sample)
NSD Sampling date not provided

NST Sampling time not provided (waters only)

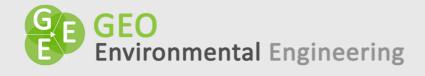
EHT Sample exceeded holding time(s)

IC Sample not received in appropriate containers
HP Headspace present in sample container

NCF Sample not chemically fixed (where appropriate)

OR Other (specify)

Lab ref	Sample id	Depth (m)	Deviating	Tests (Reason for deviation)
94487-1	RBH01	0.00-0.30	N	
94487-2	RBH01	0.50-1.00	N	
94487-3	RBH01	1.50-1.90	N	



**End of Report** 

**Website:** www.geoenvironmentalengineering.com **Email:** info@geoenvironmentalengineering.com

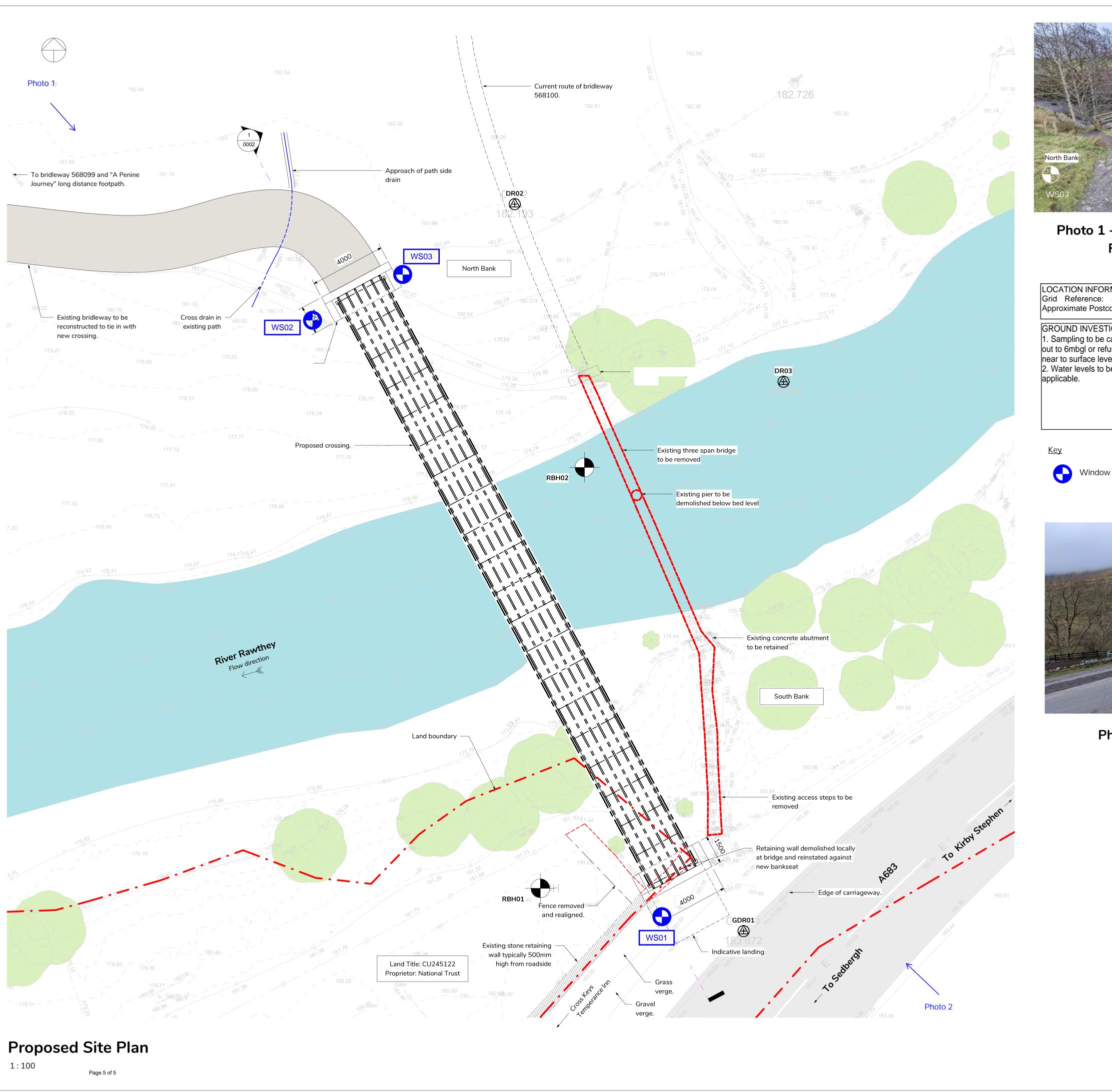




Photo 1 - View Of Existing Bridge From North Bank

LOCATION INFORMATION: Grid Reference: SD 698 969 Approximate Postcode: LA10 5NE

GROUND INVESTIGATION SPECIFICATION

1. Sampling to be carried out to BS 5930:2015. To be carried out to 6mbgl or refusal. SPT values to be obtained and as near to surface level as practical following hand dig.

2. Water levels to be suitably logged at encountered levels as applicable.

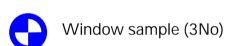




Photo 2 - View Of South Approach to Bridge

P01	Approval		AK	ARH	31.01
Rev	Description		Chkd	Ву	Date
The V Cartm Harles Shrev	er Bridges Ltd Varehouse, nel Drive, scott, vsbury SY1 3TB 1743 811 811	BEAV BRI	/EI	G	ES
Client <b>Yorl</b>	kshire Dales NPA				

Drawing Title GI Spec

Designed Checked **Drawing Status** Page Size INFORMATION Drawing No BB1543-01-XX-SK-0001 Rev P01



### **TEST REPORT**

**Client** Beaver Bridges Ltd

**Address** The Warehouse

Cartmel Drive Harlescott Shrewsbury SY1 3TB

**Contract** Cross Keys Footbridge, Cumbria LA10 5NE

Job Number MRN 4733

Date of Issue 06 April 2023

Pages 1 of 5

Approved Signatories

S J Hutchings, O P Davies

#### Notes

- 1 All remaining samples and remnants from this contract will be disposed 28 days from the date of this report unless you notify us to the contrary.
- 2 Result certificates, in this report, not bearing a UKAS mark, are not included in our UKAS accreditation schedule.
- 3 Opinions and interpretations expressed herein are outside the scope of our UKAS accreditation.
- 4 Certified that the samples have been examined and tested in accordance with the terms of the contract/order and unless otherwise stated conform to the standards/specifications quoted.
- 5 The results included within the report are representative of the samples submitted for analysis.
- 6 This certificate should not be reproduced, except in full, without the express permission of the laboratory.



Andrew House, Hadfield Street, Dukinfield, Cheshire SK16 4QX Tel: 0161 475 0870 Email: enquiries@murrayrix.com Website: www.murrayrix.com

Also at: London: 020 8523 1999

#### 1 HOLE NO. BOREHOLE LOG - MURRAY RIX GEOTECHNICAL Sheet 1 of 1 CLIENT SITE Beaver Bridges Ltd Cross Keys Footbridge, Cumbria LA10 5NE **SCALE** LEVEL/POSITION **OPERATOR** LOGGED BY JOB NO. DATE OF FIELDWORK 06/04/23 - 06/04/23 MRN 4733 1:50 WS01 PA/TW PΑ SPT N SAMPLE RECORD Standp/ **DESCRIPTION OF STRATUM (thickness)** DEPTH LEGEND (Cu-kN/m<sup>2</sup>) DEPTH TYPE Piezo Grass over TOPSOIL (0.08) Firm to stiff dark brown silty slightly gravelly CLAY. Gravel was sub-angular fine (0.73)0.08 0.50 16 D 0.80 Firm orange brown silty sandy slightly gravelly CLAY. Gravel was sub-angular fine to medium (0.90)\_1.00 D 13 1.50 D 14 1.70 Firm moist orange brown silty sandy slightly gravelly CLAY. Gravel was rounded fine (1.10)2.00 D 14 2.50 D 15 Borehole terminated due to obstruction. Lead driller believes this to be the interface of the underlying Coniston Group 2.80 D REFUSAL 2.80 (Sandstone) Bedrock. GROUNDWATER AND CASING INFORMATION BORING METHOD AND REMARKS DEPTH CASED WATER LEVEL ELAPSED TIME Limited access continuous flight auger rig. REMARKS ON GROUNDWATER AND CASING Drillers log. N/A N/A N/A N/A N/A No Groundwater encountered. All dimensions are in metres unless otherwise stated

BOI	REH	OLE	LOG	- [	MURF	RAY RIX GEC	TECHNI	CAL	HOLE NO. Sheet 1 of 1	2	
CLIENT	-		Beav	rer I	Bridges L	td	SITE	s Keys Footbridg	ge, Cumbria LA10	5NE	
DATE OF FIELDWORK         SCALE         LEVEL/POSITION           06/04/23 - 06/04/23         1:50         WS02			2	OPERATOR PA/TW	LOGGED BY	JOB NO.					
SAM DEPT	PLE RE	ECORD TYPE	SPT N (Cu-kN/r	۷ m <sup>2</sup> )	Standp/ Piezo	DESC	RIPTION OF STR	RATUM (thickness)		DEPTH	LEGEND
0.50		D	9		1	Grass over Topsoil Firm orange brown s was sub-angular fir	silty sandy sl	ightly gravelly	CLAY. Gravel	0.07	x ° · · · · · · · · · · · · · · · · · ·
1.00		D D	12 REFUSA			Borehole terminated this to be the into (Sandstone) Bedrock	erface of the	uction. Lead dri	ller believes ton Group	1.20	· · · · · · · · · · · · · · · · · · ·
GROUN DEPTH STRUCK	DWATE DEPTH CASED	ELAPSED TIME		DEPT SEALE	RMATION	REMARKS ON GROUNDWATER A		BORING METHOD Hand augered box			
N/A	N/A	N/A	N/A	N/F		roundwater encountered	d.	Drillers log. SPT values shown	n in this log are		

### 3 HOLE NO. BOREHOLE LOG - MURRAY RIX GEOTECHNICAL Sheet 1 of 1 CLIENT SITE Cross Keys Footbridge, Cumbria LA10 5NE Beaver Bridges Ltd DATE OF FIELDWORK SCALE LEVEL/POSITION OPERATOR LOGGED BY JOB NO. 06/04/23 - 06/04/23 PA/TW MRN 4733 1:50 WS03 SAMPLE RECORD SPT N Standp/ **DESCRIPTION OF STRATUM (thickness)** DEPTH LEGEND Cu-kN/m<sup>2</sup> DEPTH TYPE Piezo 0.07 Grass over Topsoil (0.07) Firm orange brown silty sandy slightly gravelly CLAY. Gravel was sub-angular fine to medium (1.34)0.50 10 D 1.00 D 12 Borehole terminated due to obstruction. Lead driller believes this to be the interface of the underlying Coniston Group 1.40 D REFUSAL 1.40 (Sandstone) Bedrock. GROUNDWATER AND CASING INFORMATION BORING METHOD AND REMARKS DEPTH CASED WATER LEVEL ELAPSED TIME Hand augered borehole. REMARKS ON GROUNDWATER AND CASING Drillers log. N/A N/A N/A N/A N/A No Groundwater encountered. SPT results shown in this log are equlivalent SPT values calculated from Mackintosh Probe Test.



### **TEST REPORT**

**Client** Beaver Bridges Ltd

**Address** The Warehouse

Cartmel Drive Harlescott Shrewsbury SY1 3TB

Contract Cross Keys Footbridge, Cumbria LA10 5NE

Job Number MRN 4733/1 **Date of Issue** 22 May 2023 **Pages** 1 of 11

Approved Signatories

S J Hutchings, O P Davies

#### Notes

- 1 All remaining samples and remnants from this contract will be disposed 28 days from the date of this report unless you notify us to the contrary.
- 2 Result certificates, in this report, not bearing a UKAS mark, are not included in our UKAS accreditation schedule.
- 3 Opinions and interpretations expressed herein are outside the scope of our UKAS accreditation.
- 4 Certified that the samples have been examined and tested in accordance with the terms of the contract/order and unless otherwise stated conform to the standards/specifications quoted.
- 5 The results included within the report are representative of the samples submitted for analysis.
- 6 This certificate should not be reproduced, except in full, without the express permission of the laboratory.



Andrew House, Hadfield Street, Dukinfield, Cheshire SK16 4QX Tel: 0161 475 0870 Email: enquiries@murrayrix.com Website: www.murrayrix.com

Also at: London: 020 8523 1999

ANDREW HOUSE, HADFIELD STREET, DUKINFIELD, CHESHIRE SK16 4QX TEL 0161 475 0870

#### **TEST CERTIFICATE**

#### PARTICLE SIZE DISTRIBUTION

BS EN ISO 17892-4:2016

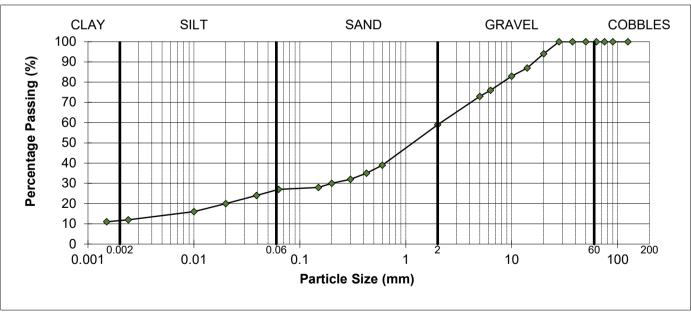
Determination of Water Content in accordance with BS EN ISO 17892-1:2014 (Oven Dry)

CLIENT	Beaver Bridges Ltd
SITE	Cross Keys Footbridge, Cumbria LA10 5NE
JOB NUMBER	MRN 4733/1

SAMPLE LABEL	BH 1 0.5, 1.0 & 1.5m	DATE SAMPLED	06-Apr-23
LAB SAMPLE No	123240, 123241, 123242	DATE RECEIVED	11-Apr-23
DATE TESTED	15-May-23	SAMPLED BY	Murray Rix

MATERIAL	See Remarks
ADVISED SOURCE	Site Investigation Sample

Sieve Size	% Passing	Specification	Sieve Size	% Passing	Specification
(mm)	(%)	(%)	(mm)	(%)	(%)
125	100		2	59	
90	100		0.6	39	
75	100		0.425	35	
63	100		0.3	32	
50	100		0.2	30	
37.5	100		0.15	28	
28	100		0.063	27	
20	94		0.039	24	
14	87		0.02	20	
10	83		0.01	16	
6.3	76		0.0024	12	
5	73		0.0015	11	



#### REMARKS

Combined sample - BH 1 0.5, 1.0 and 1.5m

Sample at 0.5m was Firm to stiff dark brown silty sandy gravelly Clay. Gravel was sub-angular fine to medium Sample at 1.0 and 1.5m was Firm orange brown silty sandy gravelly Clay. Gravel was sub-angular fine to medium

SIGNED

NAME S.J. Hutchings DATE 22-May-23 (Director)

Page 2 of 11

ANDREW HOUSE, HADFIELD STREET, DUKINFIELD, CHESHIRE SK16 4QX TEL 0161 475 0870

#### **TEST CERTIFICATE**

#### PARTICLE SIZE DISTRIBUTION

BS EN ISO 17892-4:2016

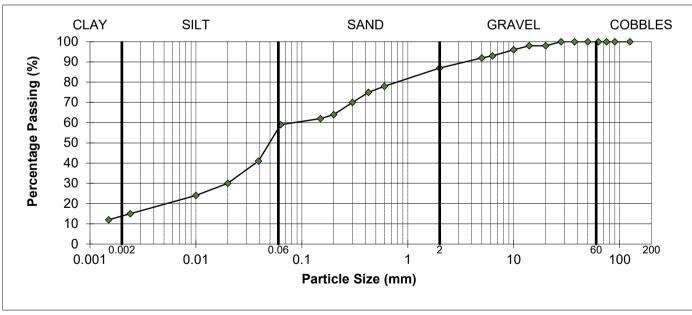
Determination of Water Content in accordance with BS EN ISO 17892-1:2014 (Oven Dry)

	- (- ))
CLIENT	Beaver Bridges Ltd
SITE	Cross Keys Footbridge, Cumbria LA10 5NE
JOB NUMBER	MRN 4733/1

SAMPLE LABEL	BH 1 2.0 & 2.5m	DATE SAMPLED	06-Apr-23
LAB SAMPLE No	123243 & 123244	DATE RECEIVED	11-Apr-23
DATE TESTED	15-May-23	SAMPLED BY	Murray Rix

MATERIAL	See Remarks
ADVISED SOURCE	Site Investigation Sample

Sieve Size	% Passing	Specification	Sieve Size	% Passing	Specification
(mm)	(%)	(%)	(mm)	(%)	(%)
125	100		2	87	
90	100		0.6	78	
75	100		0.425	75	
63	100		0.3	70	
50	100		0.2	64	
37.5	100		0.15	62	
28	100		0.063	59	
20	98		0.039	41	
14	98		0.02	30	
10	96		0.01	24	
6.3	93		0.0024	15	
5	92		0.0015	12	



### **REMARKS**

Combined sample - BH 1 2.0 and 2.5m

Firm moist orange brown silty slightly sandy slightly gravelly CLAY. Gravel was rounded fine to medium

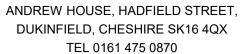
SIGNED

NAME

S.J. Hutchings DATE (Director)

ATE 22-May-23

Page 3 of 11





### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

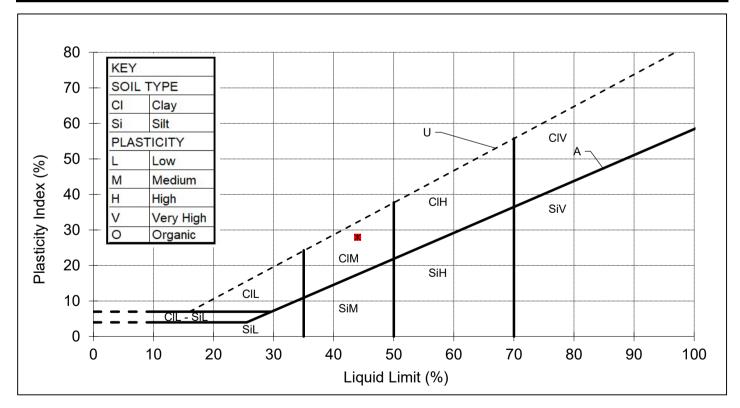
CLIENT	Beaver Bridges Ltd				
SITE	Cross Keys Footbridge, Cumbria LA10 5NE				
JOB NUMBER	MRN 4733/1				

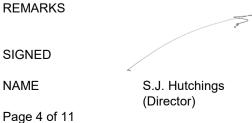
SAMPLE LABEL	BH 1 0.5m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123240	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm to stiff dark brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium				
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing		
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved		

Test Readings mm (average)		Moisture Content % Correction Factor		Correction factor
Determination 1 (avg)	21.7	45.1	0.978	from Clayton and
Determination 2 (avg)	21.2	44.8	0.976	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
9.1	44	16	28	38





DATE 22-May-23





### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

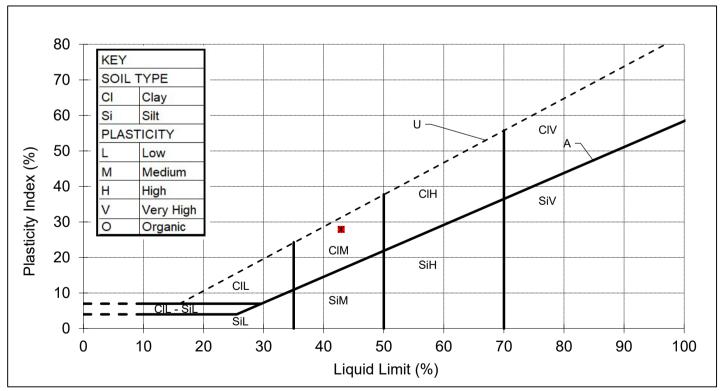
CLIENT Beaver Bridges Ltd			
SITE	Cross Keys Footbridge, Cumbria LA10 5NE		
JOB NUMBER	MRN 4733/1		

SAMPLE LABEL	BH 1 1.0m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123241	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm orange brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium			
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing	
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved	

Test Readings mm (average)		Moisture Content % Correction Factor		Correction factor
Determination 1 (avg)	19.5	42.6	1.012	from Clayton and
Determination 2 (avg)	19.3	42.2	1.012	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
9.9	43	15	28	35



REMARKS

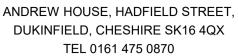
SIGNED

NAME

S.J. Hutchings (Director)

DATE 22-May-23

Page 5 of 11





### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

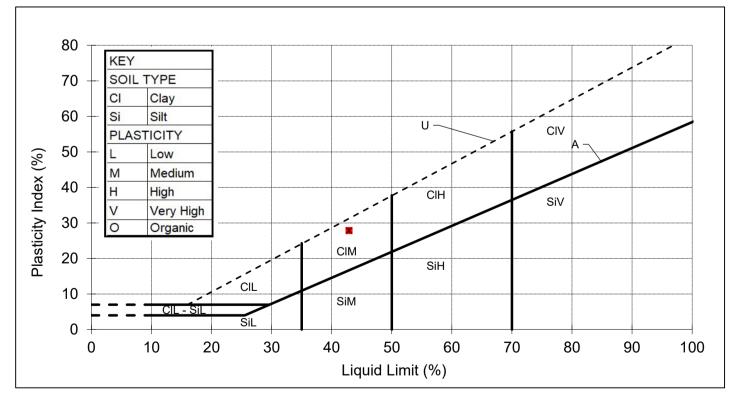
CLIENT	Beaver Bridges Ltd				
SITE Cross Keys Footbridge, Cumbria LA10 5NE					
JOB NUMBER	MRN 4733/1				

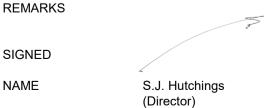
SAMPLE LABEL	BH 1 1.5m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123242	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm orange brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium			
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing	
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved	

Test Readings mm (average)		Moisture Content % Correction Factor		Correction factor
Determination 1 (avg)	19.0	42.1	1.020	from Clayton and
Determination 2 (avg)	19.0	42.0	1.020	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
9.1	43	15	28	

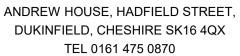




DATE

22-May-23

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### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

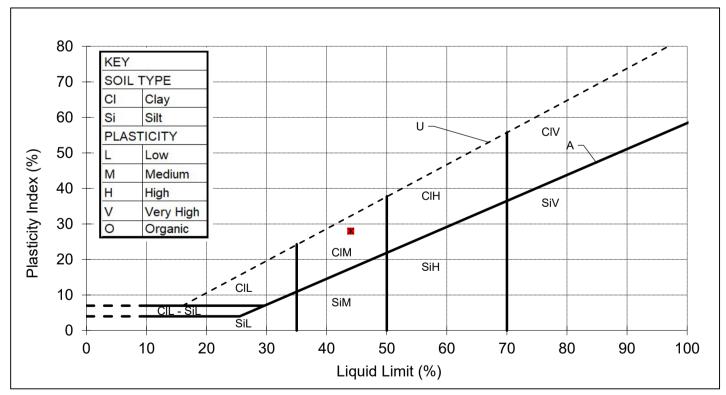
CLIENT	Beaver Bridges Ltd	
SITE	Cross Keys Footbridge, Cumbria LA10 5NE	
JOB NUMBER	MRN 4733/1	

SAMPLE LABEL	BH 1 2.0m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123243	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm moist orange brown silty slightly sandy slightly gravelly CLAY. Gravel was rounded fine to medium.		
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved

Test Readings	mm (average)	Moisture Content %	Correction Factor	Correction factor
Determination 1 (avg)	19.6	43.7	1.008	from Clayton and
Determination 2 (avg)	19.6	43.6	1.006	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
18.7	44	16	28	73



REMARKS

SIGNED

NAME

S.J. Hutchings (Director)

DATE 22-May-23

Page 7 of 11





### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

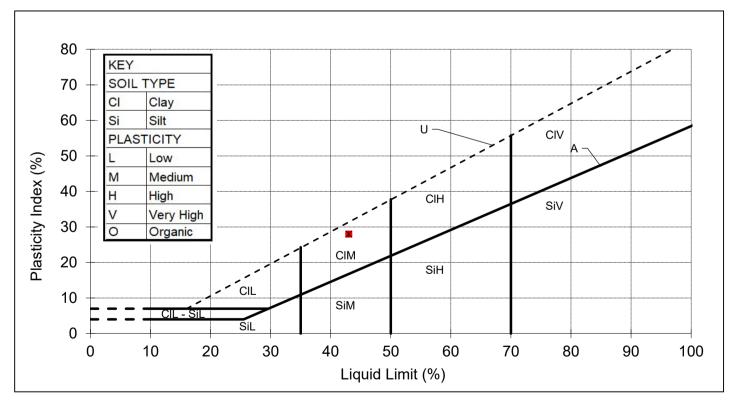
CLIENT	Beaver Bridges Ltd
SITE	Cross Keys Footbridge, Cumbria LA10 5NE
JOB NUMBER	MRN 4733/1

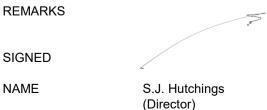
SAMPLE LABEL	BH 1 2.5m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123244	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm moist orange brown silty slightly sandy slightly gravelly CLAY. Gravel was rounded fine to medium.		
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved

Test Readings	mm (average)	Moisture Content %	Correction Factor	Correction factor
Determination 1 (avg)	20.4	43.2	0.994	from Clayton and
Determination 2 (avg)	20.4	43.3	0.994	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
19.0	43	15	28	76





DATE 22-May-23

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### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

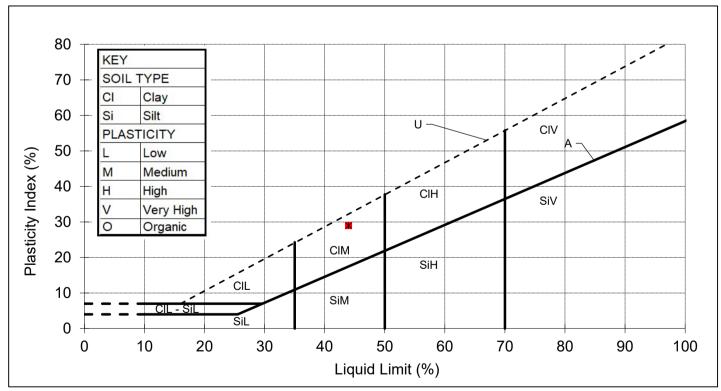
CLIENT	Beaver Bridges Ltd			
SITE	Cross Keys Footbridge, Cumbria LA10 5NE			
JOB NUMBER	MRN 4733/1			

SAMPLE LABEL	BH 2 0.5m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123245	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm orange brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium				
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing		
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved		

Test Readings	mm (average)	Moisture Content %	Correction Factor	Correction factor
Determination 1 (avg)	19.8	43.8	1.004	from Clayton and
Determination 2 (avg)	19.8	43.8	1.004	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
11.9	44	15	29	38



REMARKS

SIGNED

NAME S.J. Hutchings

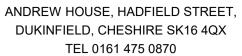
nings

(Director)

Page 9 of 11

DATE

22-May-23





### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

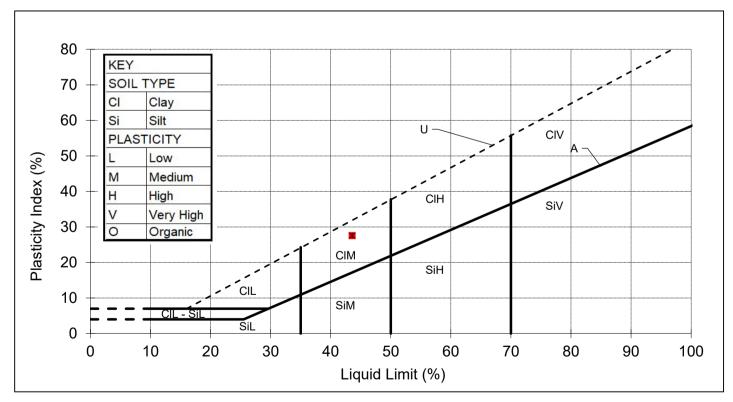
CLIENT	Beaver Bridges Ltd
SITE	Cross Keys Footbridge, Cumbria LA10 5NE
JOB NUMBER	MRN 4733/1

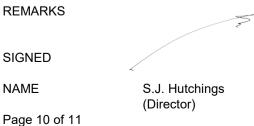
SAMPLE LABEL	BH 3 0.5m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123246	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm orange brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium			
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing	
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved	

Test Readings	mm (average)	Moisture Content %	Correction Factor	Correction factor
Determination 1 (avg)	20.2	43.6	0.997	from Clayton and
Determination 2 (avg)	20.3	43.8	0.997	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing
Content (%)	(%)	(%)	(%)	425 micron (%)
10.8	44	16	28	





DATE 22-May-23





### **TEST CERTIFICATE**

LIQUID LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.3 (30° FALL CONE) 1 POINT METHOD PLASTIC LIMIT BS EN ISO 17892-12:2018+A1:2021 Clause 5.5

WATER CONTENT METHOD BS EN ISO 17892-1:2014

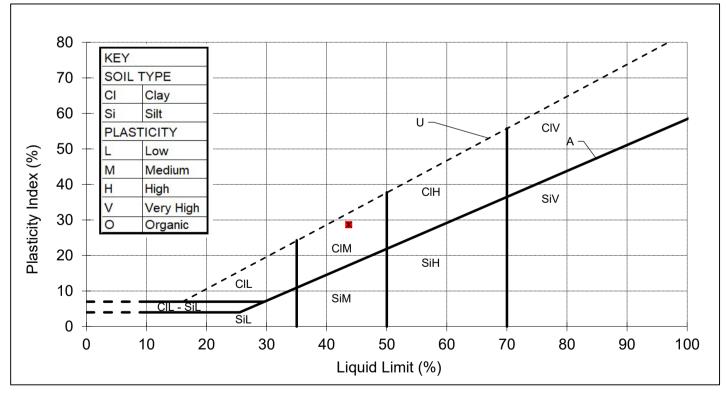
CLIENT	Beaver Bridges Ltd
SITE	Cross Keys Footbridge, Cumbria LA10 5NE
JOB NUMBER	MRN 4733/1

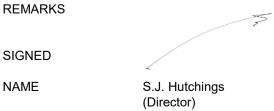
SAMPLE LABEL	BH 3 1.0m	DATE SAMPLED	06-Apr-23
SAMPLE No.	123247	DATE RECEIVED	11-Apr-23
DATE TESTED	12-Apr-23	SAMPLED BY	Murray Rix

MATERIAL	Firm orange brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium			
ADVISED SOURCE	Site Investigation Sample	WATER CONTENT	Increasing	
SAMPLE HISTORY	Natural State	% RET. 425um BY	Wet Sieved	

Test Readings	mm (average)	Moisture Content %	Correction Factor	Correction factor
Determination 1 (avg)	20.9	44.2	0.986	from Clayton and
Determination 2 (avg)	20.9	44.4	0.986	Jukes 1978

Natural Moisture	Liquid Limit	Plastic Limit	Plasticity Index	Passing	
Content (%)	(%)	(%)	(%)	425 micron (%)	
11.5	44	15	29		





DATE 22-May-23

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### **TEST REPORT**

**Client** Beaver Bridges Ltd

**Address** The Warehouse

Cartmel Drive Harlescott Shrewsbury SY1 3TB

**Contract** Cross Keys Footbridge, Cumbria LA10 5NE

Job Number MRN 4733

Date of Issue 22 May 2023

Pages 1 of 5

Approved Signatories

S J Hutchings, O P Davies

#### Notes

- 1 All remaining samples and remnants from this contract will be disposed 28 days from the date of this report unless you notify us to the contrary.
- 2 Result certificates, in this report, not bearing a UKAS mark, are not included in our UKAS accreditation schedule.
- 3 Opinions and interpretations expressed herein are outside the scope of our UKAS accreditation.
- 4 Certified that the samples have been examined and tested in accordance with the terms of the contract/order and unless otherwise stated conform to the standards/specifications quoted.
- 5 The results included within the report are representative of the samples submitted for analysis.
- 6 This certificate should not be reproduced, except in full, without the express permission of the laboratory.



Andrew House, Hadfield Street, Dukinfield, Cheshire SK16 4QX Tel: 0161 475 0870 Email: enquiries@murrayrix.com Website: www.murrayrix.com

Also at: London: 020 8523 1999

#### 1 HOLE NO. BOREHOLE LOG - MURRAY RIX GEOTECHNICAL Sheet 1 of 1 CLIENT SITE Beaver Bridges Ltd Cross Keys Footbridge, Cumbria LA10 5NE **SCALE** LEVEL/POSITION **OPERATOR** LOGGED BY DATE OF FIELDWORK JOB NO. 06/04/23 - 06/04/23 MRN 4733 1:50 WS01 PA/TW PΑ SPT N SAMPLE RECORD Standp/ **DESCRIPTION OF STRATUM (thickness)** DEPTH LEGEND (Cu-kN/m<sup>2</sup>) DEPTH TYPE Piezo Grass over TOPSOIL (0.08)Firm to stiff dark brown silty sandy gravelly CLAY. Gravel was sub-angular fine to medium (0.73)0.08 0.50 16 D 0.80 Firm orange brown silty sandy gravelly CLAY. Gravel was subangular fine to medium (0.90)\_1.00 D 13 1.50 D 14 1.70 Firm moist orange brown silty slightly sandy slightly gravelly CLAY. Gravel was rounded fine to medium (1.10)2.00 D 14 2.50 D 15 Borehole terminated due to obstruction. Lead driller believes this to be the interface of the underlying Coniston Group 2.80 D REFUSAL 2.80 (Sandstone) Bedrock. GROUNDWATER AND CASING INFORMATION BORING METHOD AND REMARKS DEPTH CASED WATER LEVEL ELAPSED TIME Limited access continuous flight auger rig. REMARKS ON GROUNDWATER AND CASING Drillers log. N/A N/A N/A N/A N/A No Groundwater encountered. All dimensions are in metres unless otherwise stated

BOREHOLE LOG - MURRAY RIX GEOTECHNICAL    HOLE NO.     Sheet 1 of 1										2	
CLIENT  Beaver Bridges Ltd  SITE  Cross Keys Footbridge, Cumbri									•	5NE	
DATE OF FIELDWORK SCALE 06/04/23 - 06/04/23 1:50						LEVEL/POSITION OPERATOR WS02 PA/TW		LOGGED BY	JOB NO. MRN 4733		
SAMPLE RECORD SPT N Standp/ DEPTH TYPE (Cu-kN/m²) Piezo		Standp/ Piezo	DESCRIPTION OF STRATUM (thickness)			DEPTH	LEGEND				
0.50		D	9			Grass over Topsoil Firm orange brown s angular fine to med	silty sandy gr	avelly CLAY. Gra	ivel was sub-	0.07	X
1.00 - 1.20 - 1.20		D D	12 REFUSA			Borehole terminated this to be the inte (Sandstone) Bedrock	erface of the k.	underlying Conis	ston Group	1.20	
DEPTH STRUCK	JNDWATER AND CASING INFORMATION  H DEPTH CASED TIME WATER DEPTH SEALED REMARKS ON GROUNDWATER.				EMARKS ON GROUNDWATER		BORING METHOD AND REMARKS Hand augered borehole.				
N/A	N/A	N/A	N/A	N/A		coundwater encountered	d.	Drillers log.  SPT values shown in this log are equlivale SPT values calculated from Mackintosh Prob Test.			

### 3 HOLE NO. BOREHOLE LOG - MURRAY RIX GEOTECHNICAL Sheet 1 of 1 CLIENT SITE Cross Keys Footbridge, Cumbria LA10 5NE Beaver Bridges Ltd DATE OF FIELDWORK SCALE LEVEL/POSITION OPERATOR LOGGED BY JOB NO. 06/04/23 - 06/04/23 1:50 PA/TW MRN 4733 WS03 SAMPLE RECORD SPT N Standp/ **DESCRIPTION OF STRATUM (thickness)** DEPTH LEGEND Cu-kN/m<sup>2</sup> DEPTH TYPE Piezo 0.07 Grass over Topsoil (0.07) Firm orange brown silty sandy gravelly CLAY. Gravel was subangular fine to medium (1.34)0.50 10 D 1.00 D 12 Borehole terminated due to obstruction. Lead driller believes this to be the interface of the underlying Coniston Group 1.40 D REFUSAL 1.40 (Sandstone) Bedrock. GROUNDWATER AND CASING INFORMATION BORING METHOD AND REMARKS DEPTH CASED WATER LEVEL ELAPSED TIME Hand augered borehole. REMARKS ON GROUNDWATER AND CASING Drillers log. N/A N/A N/A N/A N/A No Groundwater encountered. SPT results shown in this log are equlivalent SPT values calculated from Mackintosh Probe Test.